# MARIGOLD AVENUE SAFETY STUDY 

## Study Intersections:

Marigold Avenue and Peabody Road Marigold Avenue and Laurel Avenue Marigold Avenue and San Miguel Road Marigold Avenue and San Lorenzo Road

Osceola County, Florida

Prepared for:

# THE FLORIDA DEPARTMENT OF TRANSPORTATION DISTRICT 5 TRAFFIC OPERATIONS 

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DeLand, Florida 32720


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## Executive Summary

At the request of the Florida Department of Transportation (FDOT), England-Thims \& Miller, Inc. (ETM) conducted a safety study at the Marigold Avenue intersections with Peabody Road, Laurel Avenue, San Miguel Road, and San Lorenzo Road in Osceola County, Florida. The purpose of the study was to identify any improvements that address existing crash patterns and potentially reduce the frequency and severity of future crashes.

From January 1, 2012 to March 31, 2019, there were 107 collisions reported at the study intersections. These crashes resulted in no fatalities, 149 injuries (in 70 injury crashes), and $\$ 857,055$ in estimated property damage. The predominant crash types include 32 (30\%) angle, 24 ( $22 \%$ ) left-turn crashes, and $24(22 \%)$ rear-end collisions. There were also 3 pedestrian-/bicyclist-related crashes.

Eight-hour turning-movement counts were collected at the study intersections. These locations were counted by Osceola County on May 22, 2019 (at the San Lorenzo Road and Peabody Road intersections) and on August 27, 2019 (at the San Miguel Road and Laurel Avenue intersections). ETM personnel completed field observations during the AM and PM peak periods as well as the off-peak hours to identify existing operational deficiencies and potential safety issues.

To reduce crash frequencies and improve the overall operations within the corridor, constructing roundabouts at the significant Marigold Avenue intersections (Peabody Road, Laurel Avenue, and San Lorenzo Road) is recommended (please see the Long-Term Concept Diagrams located in Appendix G).

Roundabouts feature channelized approaches and a center island that results in lower speeds and fewer conflict points. By constructing roundabouts at the study intersections, the corridor can anticipate safety and operational benefits. The intersections would operate more efficiently and the number of crashes (and the crash severity) should be reduced. This corridor-wide improvement should also have a traffic-calming effect, slowing traffic and creating a more pedestrian-friendly environment.

The following long-term improvements are recommended:

- Construct a roundabout at the Peabody Road/Marigold Avenue intersection.
- Construct a roundabout at the Laurel Avenue/Marigold Avenue intersection.
- Construct a northbound left-turn bay at the San Miguel Road/Marigold Avenue intersection.
- Construct a roundabout at the San Lorenzo Road/Marigold Avenue intersection.

A benefit-to-cost analysis was calculated for the proposed improvements and was based on criteria outlined in the Highway Safety Improvement Program Manual. The estimated cost to provide these improvements is $\$ 6,157,741$ (which reflects an annual cost of $\$ 461,795$ ). The resulting annual benefit is expected to be $\$ 1,059,871$. The calculated benefit/cost ratio is $\mathbf{2 . 3 0}$ and the Net Present Value is \$6,647,354.
Additionally, the following low-cost, short-term improvements are recommended:

- Replace the school crossing signs with fluorescent yellow-green school crossing signs and supplemental plaques. Also, re-stripe the existing "SCHOOL" pavement messages/markings.
- Install high-emphasis crosswalk striping at all existing marked crosswalks at the study intersections.
- Install "STOP" pavement messages and additional STOP signs to supplement some existing STOP signs. Also, install red "bright sticks" to enhance the visibility of some existing STOP signs.
- Re-stripe the double yellow centerline and replace the Y/Y RPMs at each study intersection.

The estimated cost for these short-term improvements is $\$ 74,823$.

## Introduction

England-Thims \& Miller, Inc. (ETM) performed a safety study at the Marigold Avenue intersections with Peabody Road, Laurel Avenue, San Miguel Road, and San Lorenzo Road in Osceola County, Florida. The purpose of the study was to identify improvements that could reduce the frequency and/or severity of crashes.

The analysis methods used in completing this study are consistent with the Federal Highway Administration's Manual on Uniform Traffic Control Devices (MUTCD 2009), the American Association of State Highway and Transportation Officials' (AASHTO) Highway Safety Manual (2010), FDOT's Traffic Engineering Manual (TEM January 2019), FDOT's Manual of Uniform Traffic Studies (MUTS 2016), FDOT's Median Handbook (2014), and FDOT's Design Manual (January 2019). This report contains existing conditions, collision analyses, qualitative assessments, recommended improvements, cost estimates and final recommendations.


Figure 1 - Project Location/Vicinity Map

## Existing Conditions

Table 1 summarizes the existing conditions for the study corridor. The conditions diagrams are provided in Appendix A. Site photos are provided in Appendix B.

Table 1 - Existing Conditions

| Feature | Description |
| :---: | :---: |
| Roadway | Marigold Avenue |
| Area Location | This corridor is located in Kissimmee, Florida (Osceola County) |
| Surrounding <br> Development | The corridor serves a predominantly residential area. |
| Marigold Avenue | - Cross Section - two-lane, undivided rural typical section <br> - Posted Speed Limit - 45 mph (except for 30 mph from approximately $500^{\prime}$ south of Laurel Avenue to about 1500 ' north of Laurel Avenue) <br> - $\quad$ AADT $-5,500 \mathrm{vpd}$ with a daily T-factor of $1.9 \%$ <br> - Sidewalks exist on the west side of the corridor (between Peabody Road and San Lorenzo Road) <br> - Street lighting exists along the east side of Marigold Avenue |
| Peabody Road at Marigold Avenue | - Intersection - Cross-intersection (four legs) <br> - Cross Section - two-lane, undivided rural typical section <br> - Posted Speed Limit - 30 mph <br> - Traffic Control - All-way stop-controlled |
| Laurel Avenue at Marigold Avenue | - Intersection - Cross-intersection (four legs) <br> - Cross Section - two-lane, undivided rural typical section <br> - Posted Speed Limit - 30 mph <br> - Traffic Control - Side-street stop-controlled |
| San Miguel Road at Marigold Avenue | - Intersection - T-intersection (three legs) <br> - Cross Section - two-lane, undivided rural typical section <br> - Posted Speed Limit - 30 mph <br> - Traffic Control - Side-street stop-controlled |
| San Lorenzo Road at Marigold Avenue | - Intersection - Cross-intersection (four legs) <br> - Cross Section - two-lane, undivided rural typical section <br> - Posted Speed Limit - 30 mph <br> - Traffic Control - Side-street stop-controlled |

## Traffic Conditions

There is one FDOT Traffic Count Station on Marigold Avenue (just north of Peabody Road). Historical traffic data and traffic characteristics are shown in Table 2.

Table 2 - Historical Local Traffic Data and Characteristics

| Characteristics | FDOT Count Station <br> 927085 <br> 22rth of Koa Street | AADT Flags |
| :---: | :---: | :---: |
| 2014 AADT | 7,800 | Computed |
| 2015 AADT | 8,000 | First Year Estimate |
| 2016 AADT | 8,200 | Second Year Estimate |
| 2017 AADT | 5,300 | Computed |
| 2018 AADT | 5,500 | First Year Estimate |
| 2018 K-Factor | 9.00 |  |
| 2018 D-Factor | 53.60 |  |
| 2018 T-Factor | 1.90 |  |

A review of the 24 -hour counts taken by Osceola County helped determine times for the morning, afternoon and evening peaks. Traffic data was collected from 6:30-9:00 AM and 2:00-7:30 PM. Data collected included 8 -hour turning-movement counts and pedestrian/bicycle activity at the following locations along Marigold Avenue:

- Peabody Road
- Laurel Avenue
- San Miguel Road
- San Lorenzo Road

All traffic data collected as part of this study is included in Appendix J.

The AM/PM peak-hour turning-movement counts are summarized in Figure 2 (shown on the next page).

## Collision Analysis

Within the study area, crash data was obtained from FDOT's Crash Analysis Reporting System (CARS) and the University of Florida's Signal Four Analytics for the seven-year (plus) period between January 1, 2012 and March 31, 2019.

During this time, a total of 107 crashes were reported at the study intersections and included the following crash types:

- 32 angle
- 4 sideswipe
- 24 left-turn
- 3 pedestrian/bicyclist
- 24 rear-end
- 10 off-road/rollover
- 5 other
- 3 right-turn
- 1 head-on
- 1 animal


An annual summary of the crash types (Table 3), crash severities (Table 4), and lighting/roadway conditions (Table 5) are shown below:

Table 3: Crash Types by Year

| Year | $\mathbf{2 0 1 2}$ | $\mathbf{2 0 1 3}$ | $\mathbf{2 0 1 4}$ | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | Jan - Mar <br> $\mathbf{2 0 1 9}$ | SUM |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type |  |  |  |  |  |  |  |  |  |
| Angle | 6 | 2 | 5 | 2 | 4 | 6 | 6 | 1 | 32 |
| Animal | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Bicycle | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| Head On | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 |
| Left Turn | 5 | 3 | 1 | 1 | 3 | 6 | 5 | 0 | 24 |
| Off Road | 2 | 0 | 2 | 1 | 1 | 1 | 0 | 0 | 7 |
| Other | 2 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 5 |
| Pedestrian | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 2 |
| Rear End | 5 | 4 | 1 | 1 | 2 | 5 | 6 | 0 | 24 |
| Right Turn | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 3 |
| Rollover | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 3 |
| Sideswipe | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 4 |
| Unknown | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 23 | 11 | 11 | 6 | 13 | 21 | 20 | 2 | $\mathbf{1 0 7}$ |

Table 4: Crash Severity by Year

| Year | $\mathbf{2 0 1 2}$ | $\mathbf{2 0 1 3}$ | $\mathbf{2 0 1 4}$ | $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 6}$ | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | Jan - Mar <br> $\mathbf{2 0 1 9}$ | SUM |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Type |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Fatal | 0 | 2 | 5 | 1 | 3 | 4 | 2 | 0 | 18 |
| Incapacitating | 1 | 2 | 0 | 2 | 4 | 2 | 5 | 1 | 22 |
| Non-Incapacitating | 7 | 1 | 0 | 2 | 0 | 0 |  |  |  |
| Possible | 5 | 3 | 4 | 2 | 1 | 10 | 5 | 0 | 30 |
| Property Damange Only | 10 | 5 | 2 | 1 | 5 | 5 | 8 | 1 | 37 |
| TOTAL | 23 | 11 | 11 | 6 | 13 | 21 | 20 | 2 | $\mathbf{1 0 7}$ |

Table 5: Lighting and Roadway Condition by Year

| Year | 2012 | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 | $\begin{gathered} \text { Jan - Mar } \\ 2019 \end{gathered}$ | SUM |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lighting Condition |  |  |  |  |  |  |  |  | 107 |
| Daylight | 18 | 8 | 7 | 5 | 11 | 16 | 13 | 0 | 78 |
| Dark - Lighted | 3 | 2 | 3 | 0 | 0 | 2 | 2 | 0 | 12 |
| Dark - Not Lighted | 2 | 1 | 1 | 1 | 1 | 2 | 3 | 1 | 12 |
| Dark - Unknown Lighting | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Dusk/Dawn | 0 | 0 | 0 | 0 | 1 | 1 | 2 | 1 | 5 |
| Unknown | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Roadway Condition |  |  |  |  |  |  |  |  | 107 |
| Dry Pavement | 17 | 8 | 9 | 6 | 12 | 16 | 18 | 2 | 88 |
| Wet Pavement | 6 | 3 | 2 | 0 | 1 | 5 | 2 | 0 | 19 |
| Mud, Dirt, Gravel | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Unknown | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |

Additional collision information:

- 70 crashes ( $65 \%$ ) resulted in 149 injuries.
- There were 19 ( $18 \%$ ) wet-pavement crashes and 29 ( $27 \%$ ) nighttime crashes, which included dawn and dusk.
- The total property damage amount for all 107 crashes was estimated to be $\$ 857,055$.

Detailed crash summaries for each intersection and collision diagram are located in Appendix C and Appendix D, respectively.

## Qualitative Assessment

To assess the existing operating conditions and determine what, if any, improvements could be recommended (to enhance safety and efficiency for the traveling public), the study corridor was observed by a registered professional engineer.

Observations occurred primarily during peak hours derived from the intersection turning movement counts.

## General Observations

- Numerous school buses were observed throughout the corridor turning into (and out from) all of the study intersections.
- The posted speed limit changes throughout the corridor (especially when considering school hours).
- Parents park on the side of road and wait on the north side of San Lorenzo Road/Marigold Avenue intersection.
- Crossing guards were stationed at several intersections along Marigold Avenue.


## Planned Improvements

Marigold Avenue between Peabody Road and San Lorenzo Road is part of the Osceola County's 2016 Safe Routes To School (SRTS) application. Sidewalk is proposed on the northeast side of Marigold Avenue from Peabody Road south to Deerwood Elementary School and at San Lorenzo Road's northernmost connection to Marigold Avenue. Please see the exhibit in Appendix E for more information.

## Intersection Evaluation

## Peabody Road and Marigold Avenue

The intersection of Peabody Road and Marigold Avenue has four legs. This intersection is currently operating with all-way STOP control.
Collisions: Between 2012 and March 2019, there were 10 left-turn/angle collisions. Four (4) of these collisions occurred in the last three years (2016-2018).

Pedestrians/Bicyclists: The south leg of Marigold Avenue is the only approach with a marked crosswalk. Sidewalk runs east/west along the south side of Peabody Road and north/south on the west side of Marigold Avenue ending at Peabody Road. During the eight hours of turning movement counts collected, 20 pedestrians and 11 bicyclists crossed Marigold Avenue at this intersection.

Operational Analysis: Synchro software was used to evaluate the corridor's existing and future traffic operations. The existing conditions analysis resulted in LOS C during the AM peak hour and LOS B during the PM peak hour. A single-lane roundabout was also analyzed, which resulted in LOS A results during the AM and PM peak hours. The volumes on Marigold Avenue were then increased by $2 \%$ annually for 21 years. In 2040, the roundabout would remain at LOS A during the peak hours. For comparison, the allway STOP-controlled intersection (in 2040) is expected to operate with LOS E results during the AM peak hour and with LOS C results during the PM peak hour.

Table 6: Peabody Road Intersection Analysis Results

| Year | Peak <br> Hour | Intersection Control |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | TWSC |  | AWSC |  | Signal |  | Roundabout |  |
|  |  | Delay | LOS* | Delay | LOS | Delay | LOS | Delay | LOS |
| 2019 | AM | 9.8 | D | 15.6 | C | 10.9 | B | 5.8 | A |
|  | PM | 5.6 | D | 12.5 | B | 9.7 | A | 5.5 | A |
| 2040 | AM | 14.5 | E | 38 | E | 12.2 | B | 7.1 | A |
|  | PM | 6.1 | E | 17.7 | C | 10.2 | B | 6.6 | A |
| *LOS was used from the approach with the greatest delay |  |  |  |  |  |  |  |  |  |

Recommendation: The four-way STOP currently functions satisfactory and the collisions are minor. A roundabout would function better at this location and could be used as a traffic-calming measure, to beautify the corridor, and to provide consistency throughout the Marigold Avenue corridor (especially if roundabouts are constructed at the San Lorenzo Road and Laurel Avenue intersections).

## Laurel Avenue and Marigold Avenue

The intersection of Laurel Avenue and Marigold Avenue has four legs. This intersection is currently operating with two-way STOP control as Laurel Avenue stops for Marigold Avenue.
Collisions: Between 2012 and March 2019, there were 20 left-turn/angle collisions. Ten (10) of these collisions occurred in the last three years (2016-2018).

Pedestrians/Bicyclists: The south leg of Marigold Avenue and the west leg of Laurel Avenue are the only approaches with marked crosswalks. Sidewalk runs east/west along the south side of Laurel Avenue, on the north side of Laurel Avenue (to the west of Marigold Avenue), and north/south on the west side of Marigold Avenue. During the eight hours of turning movement counts collected, 27 pedestrians and 22 bicyclists crossed Marigold Avenue at this intersection.

Operational Analysis: Synchro software was used to evaluate the corridor's existing and future traffic operations. The existing conditions analysis resulted in LOS C during the AM and PM peak hours. A single-lane roundabout was also analyzed, which resulted in LOS A results during the AM and PM peak hours. The volumes on Marigold Avenue were then increased by $2 \%$ annually for 21 years. In 2040, the roundabout would remain at LOS A during the peak hours. For comparison, an all-way STOP-controlled intersection (in 2040) is expected to operate with LOS D results during the AM peak hour and with LOS C results during the PM peak hour.

Table 7: Laurel Avenue Intersection Analysis Results

| Year | Peak <br> Hour | Intersection Control |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | TWSC |  | AWSC |  | Signal |  | Roundabout |  |
|  |  | Delay | LOS* | Delay | LOS | Delay | LOS | Delay | LOS |
| 2019 | AM | 5.9 | C | 13.6 | B | 9.2 | A | 5.5 | A |
|  | PM | 3.9 | C | 11.8 | B | 8.7 | A | 5.2 | A |
| 2040 | AM | 9.7 | F | 31.1 | D | 9.7 | A | 6.9 | A |
|  | PM | 4.5 | D | 18.9 | C | 9 | A | 6.4 | A |
| *LOS was used from the approach with the greatest delay |  |  |  |  |  |  |  |  |  |

Recommendation: The two-way STOP currently functions satisfactory. A roundabout would function better at this location and could be used as a traffic-calming measure, to beautify the corridor, and to provide consistency throughout the Marigold Avenue corridor (especially if roundabouts are constructed at the Peabody Road and San Lorenzo Road intersections).

## San Miguel Road and Marigold Avenue

The intersection of San Miguel Road and Marigold Avenue has three legs (San Miguel "tees" into Marigold). This intersection is currently operating with side-street STOP-control as San Miguel Road stops for Marigold Avenue.
Collisions: Between 2012 and March 2019, there were 24 collisions at/near the study intersection. Ten were northbound rear-ends and three were collisions occurring from a northbound motorist attempting to pass another motorist waiting to complete a left-turn movement onto San Miguel Road.
Pedestrians/Bicyclists: There are is one marked crosswalks at this intersection (across San Miguel). Sidewalk runs north/south on the west side of Marigold Avenue. During the eight hours of turning movement counts collected, one bicyclist crossed Marigold Avenue.
Operational Analysis: Synchro software was used to evaluate the corridor's existing and future traffic operations. The existing conditions analysis resulted in LOS A during the peak hours with an eastbound approach delay (for San Miguel Road) of 13.5 seconds during the AM peak hour and 14.2 seconds during the PM peak hour.
Recommendation: Construct a dedicated northbound left-turn bay on Marigold Avenue.

## San Lorenzo Road and Marigold Avenue

The intersection of San Lorenzo Road and Marigold Avenue has four legs. This intersection is currently operating with two-way STOP control as San Lorenzo Road stops for Marigold Avenue.

Collisions: Between 2012 and March 2019, there were 25 left-turn/angle collisions. Sixteen (16) of these crashes occurred within the last three years (2016-2018).

Pedestrians/Bicyclists: The north leg of Marigold Avenue and the west leg of San Lorenzo Road are the only approaches with marked crosswalks. Sidewalk runs east/west along the north side of San Lorenzo Road and north/south on the west side of Marigold Avenue. During the eight hours of turning movement counts collected, 38 pedestrians and 7 bicyclists crossed Marigold Avenue at this intersection.

Operational Analysis: Synchro software was used to evaluate the corridor's existing and future traffic operations. The existing conditions analysis resulted in LOS F for the southwest approach (for motorists on San Lorenzo) during the peak hours. This intersection was analyzed to determine if an all-way STOP was feasible and the Synchro analysis resulted in LOS F during the PM peak hour. Signal warrants were reviewed and a signal would be warranted based on existing volumes. A single-lane roundabout was also analyzed, which resulted in a LOS A during AM and PM peak hours. The volumes on Marigold Avenue were then increased by $2 \%$ annually for 21 years. In 2040, the roundabout would experience LOS B results during the AM and PM peak hours.

Table 8: San Lorenzo Road Intersection Analysis Results

| Year | Peak <br> Hour | Intersection Control |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | TWSC |  | AWSC |  | Signal |  | Roundabout |  |
|  |  | Delay | LOS* | Delay | LOS | Delay | LOS | Delay | LOS |
| 2019 | AM | 16 | F | 32.2 | D | 10 | B | 7 | A |
|  | PM | 9.8 | F | 50.4 | F | 9.8 | A | 7.5 | A |
| 2040 | AM | 64.7 | F | 137.7 | F | 11.8 | B | 10.1 | B |
|  | PM | 41.8 | F | 158.8 | F | 11.8 | B | 10.9 | B |
| *LOS was used from the approach with the greatest delay |  |  |  |  |  |  |  |  |  |

A partial signal warrant analysis was completed for the intersection of San Lorenzo Road and Marigold Avenue. The Manual on Uniform Traffic Control Devices (MUTCD) provides requirements and guidance for determining if signalization is the best method of traffic control. Chapter 4C of the 2009 edition (revised May 2012) of the MUTCD was used for this effort.
The collected turning-movement count data is summarized in Table 9, Warrant 1 (Eight-Hour Vehicle Volume) below. The posted speed limit in this area is 45 MPH , therefore this location would qualify for the reduction of the required approach volumes (from $100 \%$ to $70 \%$ ). As a result, the required volume for the major roadway would be 350 vehicles per hour (vph) and the corresponding required volume for the minor roadway would be 105 vph (for Condition A, Minimum Vehicular Volume). For Condition B (Interruption of Continuous Traffic), the minimum requirements would be 525 vph for the major roadway and 53 vph for the minor roadway.

Table 9 - WARRANT 1 (Eight-Hour Vehicular Volume) - Existing Volumes

|  | NUMBER OF VEHICLES |  | IS HOURLY WARRANT SATISFIED? |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Period | Marigold <br> Avenue | Minor Street <br> (WB San Lorenzo) | Condition <br> A | Condition <br> B | Combination <br> of A \& B |  |  |  |  |
| 6:30-7:00 AM | 332 | 63 | NO $* *$ | NO $* *$ | N/A |  |  |  |  |
| $7: 00-8: 00$ AM | 946 | 152 | YES | YES | N/A |  |  |  |  |
| 8:00-9:00 AM | 906 | 119 | YES | YES | N/A |  |  |  |  |
| $2: 00-3: 00$ PM | 890 | 105 | YES | YES | N/A |  |  |  |  |
| $3: 00-4: 00$ PM | 1060 | 104 | NO | YES | N/A |  |  |  |  |
| $4: 00-5: 00$ PM | 1120 | 114 | YES | YES | N/A |  |  |  |  |
| 5:00-6:00 PM | 1102 | 114 | YES | YES | N/A |  |  |  |  |
| 6:00-7:00 PM | 1081 | 115 | YES | YES | N/A |  |  |  |  |
| NUMBER OF HOURS MEETING WARRANT | 6 | 7 | N/A |  |  |  |  |  |  |
| IS WARRANT SATISFIED? |  |  |  |  |  |  | NO | NO | N/A |

[^0]A review of this data (see Table 9, previous page) shows that the required minimum volumes for Condition B are met for 7 of the 8 hours. However, based on the 6:30-7:00 AM volumes, it is reasonable to assume Warrant 1 requirements would be satisfied (if 6:00-6:30 AM or 9:00-9:30 AM data were included). For this reason, Warrant 1 is expected to be satisfied.


Figure 3 - MUTCD's Figure 4C-2, Four-Hour Vehicular Volume (70\% Factor)
Warrant 2 (Four-hour Vehicular Volume) was also examined. The MUTCD permits the use of Figure 4C2, Four-Hour Vehicular Volume ( $70 \%$ Factor) at this location. The lower threshold for the minor-street approach is 60 vehicles per hour ( vph ) when the major-street volume is equal to or exceeds 900 vph .

When the highest Table 9 volumes were plotted on Figure 4C-2, at least seven (7) hours were located on the warranted side of the "Four-Hour" curves. For this reason, the required volumes meet or exceed the minimum requirements. As a result, Warrant 2 is satisfied.

In addition, there have been eight angle/left-turn collisions in 2018. Warrant 7 (Crash Experience) can be applied in situations "where the collision severity and frequency are the principal reasons" for installing a signal. One of the requirements needed to satisfy this warrant is that five (5) or more "reported crashes, of types susceptible to correction by a traffic control signal, have occurred within a 12 -month period, each crash involving personal injury or property damage apparently exceeding the applicable requirements for a reportable crash."
Based on the warrants and crashes mentioned above, a traffic signal is warranted at the intersection of San Lorenzo Road with Marigold Avenue.

Recommendation: Although a traffic signal is warranted, a roundabout is recommended at this intersection. Based on the operational analysis results, a roundabout should work well for the next 20 -plus years and could be used as a traffic-calming measure, to beautify the corridor, and to provide consistency throughout the Marigold Avenue corridor (especially if roundabouts are constructed at the Peabody Road and Laurel Avenue intersections).

## Recommended Improvements

After reviewing the study corridor, short-term and long-term conceptual plans were developed. By constructing roundabouts at the significant intersections, the corridor should experience safety and operational benefits. The intersections should operate more efficiently and the number of crashes (and crash severities) would decrease. This corridor-wide improvement should also have a traffic-calming effect, slowing traffic and creating a more pedestrian-friendly environment.
When long-term costs are considered, roundabouts eliminate the equipment/materials/devices, maintenance and electrical costs associated with traffic signals. During power outages, roundabouts are much more effective (when compared to traffic signals). When a traditional signalized intersection does not have power, motorists must treat the intersection as a four-way stop. Another costly option is to have enforcement agencies direct traffic. However, roundabouts operate normally regardless of whether power is available or not.

In addition, there are 32 conflict points associated with a conventional four-leg intersection (eight merging or joining, eight diverging or separating, and 16 crossing). By contrast, roundabouts serving four legs only have eight conflict points (four merging and four diverging). Not only are the number of conflict points reduced with a roundabout, the conflicts that remain typically result in substantially less-severe crashes, which in turn, decreases the likelihood of injury. The reduction in both the total number of conflict points and the resulting crash severity also benefits pedestrians and bicyclists.

## Long-Term Improvements

The following long-term improvement are recommended:

- Construct a roundabout at the Peabody Road/Marigold Avenue intersection.
- Construct a roundabout at the Laurel Avenue/Marigold Avenue intersection.
- Construct a northbound left-turn bay at the San Miguel Road/Marigold Avenue intersection.
- Construct a roundabout at the San Lorenzo Road/Marigold Avenue intersection.


## Short-Term Improvements

The following low-cost, short-term improvements are also recommended:

- Replace the school crossing signs with fluorescent yellow-green school crossing signs and supplemental plaques. Also, re-stripe the existing "SCHOOL" pavement messages/markings.
- Install high-emphasis crosswalk striping at all existing marked crosswalks at the study intersections.
- Install "STOP" pavement messages and additional STOP signs to supplement some existing STOP signs. Also, install red "bright sticks" to enhance the visibility of some existing STOP signs.
- Re-stripe the double yellow centerline and replace the Y/Y RPMs at each study intersection.


## Proposed Roundabouts - Fastest Path Analyses

ETM performed additional analyses on each proposed roundabout to confirm they meet the criteria for FDM and NCHRP 672. Fastest path analyses (see Table 10, next page) indicate each approach movement is within the $20-25 \mathrm{MPH}$ range for R 1 using the criteria of a single lane roundabout.

Table 10 - Fastest Path Analyses - Proposed Roundabouts on Marigold Avenue

| Main Street | Side Street | Dir. | $\begin{gathered} \text { R1 } \\ (\mathbf{F T}) \end{gathered}$ | MPH | $\begin{gathered} \text { R2 } \\ (\mathbf{F T}) \end{gathered}$ | MPH | $\begin{gathered} \text { R3 } \\ \text { (FT) } \end{gathered}$ | MPH <br> (Measured) | $\begin{aligned} & \text { D23 } \\ & \text { (LF) } \end{aligned}$ | R3 MPH (Calculated) | $\begin{gathered} \text { R4 } \\ (\mathbf{F T}) \end{gathered}$ | MPH |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Marigold | Peabody | EB | 93.09 | 20 | 53.66 | 16 | 204.34 | 27 | 109 | 38 |  |  |
| Marigold | Peabody | WB | 165.11 | 25 | 64.1 | 17 | 392.15 | 35 | 103 | 43 |  |  |
| Peabody | Marigold | SB | 148.22 | 24 | 54.13 | 16 | 212.37 | 27 | 100 | 37 |  |  |
| Peabody | Marigold | NB | 158.79 | 24 | 44.1 | 15 | 174.96 | 25 | 102 | 36 |  |  |
| Marigold | Laurel | EB | 129.46 | 23 | 48.74 | 15 | 206.53 | 27 | 98 | 37 |  |  |
| Marigold | Laurel | WB | 150.19 | 24 | 52.56 | 16 | 318.88 | 32 | 98 | 41 |  |  |
| Laurel | Marigold | SB | 137.47 | 23 | 50.2 | 16 | 190.79 | 26 | 100 | 36 |  |  |
| Laurel | Marigold | NB | 172.06 | 25 | 50.87 | 16 | 216.17 | 27 | 99 | 37 |  |  |
| Marigold | San Lorenzo | EB | 152.17 | 24 | 50.14 | 16 | 261.93 | 30 | 108 | 40 |  |  |
| Marigold | San Lorenzo | WB | 137.84 | 23 | 63.06 | 17 | 285.87 | 31 | 111 | 40 |  |  |
| San Lorenzo | Marigold | SB | 145.69 | 24 | 39.9 | 14 | 195.45 | 26 | 108 | 37 |  |  |
| San Lorenzo | Marigold | NB | 173.85 | 25 | 65.75 | 17 | 247.17 | 29 | 101 | 38 |  |  |

## Other Considerations

The posted speed limit is 45 mph approaching San Lorenzo Road, then drops to 30 mph approximately 400 , before Laurel Avenue. The 30 mph continues for approximately 2,000' north, then increases to 45 mph . There are no apparent context classification or typical section changes within the 30 mph segment. In addition, 20 mph school zones currently exist at San Lorenzo Road, north of Laurel Avenue at the entrance to Deerwood Elementary School, and at Peabody Road. Especially during school-zone hours, the changing speed limits may create confusion. If the roundabout improvements are pursued, consideration should be given to placing one consistent speed limit throughout the corridor.

## Benefit-To-Cost Analysis

Proposed Short-Term and Long-Term Concept Diagrams are included in Appendix F and Appendix G, respectively. Cost estimates were based on FDOT's Historical Costs from 08/01/18 to 07/31/19. If available, Area 8 cost data was used; if unavailable, statewide cost data was used. The appropriate pay items and estimated quantities were used to generate an opinion of probable costs. The cost estimates are located in Appendix H.

A benefit/cost analysis was completed for the proposed long-term improvements and was based on criteria outlined in the Highway Safety Improvement Program Manual. The estimated engineering and construction costs associated with the improvements are $\$ 6,157,741$ (which reflects an annual cost of $\$ 461,795$ ). Based on the Federal Highway Administration's (FHWA's) Crash Modification Factors Clearinghouse, two crash modification factors (CMF) were identified. A crash reduction factor of $52.7 \%$ (for conversion of intersection into low-speed roundabout) was used. This was applied to 31 of the applicable crashes at the study intersections recommended to be converted into roundabouts. A reduction of $43.4 \%$ was identified for installing a left-turn lane and was applied to 8 of the applicable crashes at the Marigold Avenue and San Miguel Road intersection. These factors were applied individually to specific crashes (no collisions were used twice when applying crash modification factors).
The resulting annual benefit is expected to be $\$ 1,059,871$. The calculated benefit/cost ratio is $\mathbf{2 . 3 0}$ and the Net Present Value is $\mathbf{\$ 6 , 6 4 7 , 3 5 4}$. The associated B/C analysis forms, NPV, and CMF information (for the long-term improvements) are located in Appendix I. The recommended improvements satisfy the minimum requirements of $\mathrm{B} / \mathrm{C}>2$ and NPV $>0$, and indicate the proposed improvements are justified and should be pursued as Highway Safety Improvement Program (HSIP) funding becomes available.

## Project Documentation, Variations and Exceptions

With the proposed concept plans, no variations or exceptions are anticipated.

## APPENDIX

## Appendix A: Existing Condition Diagrams

Appendix B: Site Photos
Appendix C: Crash Summaries
Appendix D: Collision Diagrams
Appendix E: Safe Routes To School Sidewalk Improvements
Appendix F: Short-Term Concept Diagrams
Appendix G: Long-Term Concept Diagrams
Appendix H: Improvements Cost Estimates
Appendix I: B/C Analysis, Net Present Value, and Crash Modification Factors
Appendix J: Turning Movement Volumes

## Appendix A:

## Existing Condition Diagrams






## Appendix B:

Site Photos

## Marigold Avenue / Peabody Road



Northbound


Eastbound


Southbound


Westbound

## Marigold Avenue / Laurel Avenue



Northbound



Southbound


## Marigold Avenue / San Miguel Road



Northbound


Southbound


Marigold Avenue / San Lorenzo Road


Northbound


Eastbound


Southbound


Westbound

## Appendix C:

## Crash Summaries




Form 750-020-06


| State of Florida Department of Transportation COLLISION SUMMARY |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| General Information |  |  |  |  |  |  |  |  |  |  |  |  |
| Section/Roadway ID: <br> Intersecting Route: <br> Milepost: <br> County: |  | Laurel Avenue |  |  |  | State Road: <br> Study Period: <br> Data by: <br> Date: |  |  | Marigold Avenue |  |  |  |
|  |  | 1/1/1 |  | To: |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  | ETM |  |
|  |  | Osceola |  |  | 0/2019 |  |  |  |  |
| No. | Date |  |  |  |  | Day | Time | Severity |  | Property Damage | Crash Type |  | Day / Night | Wet / Dry | Contributing Cause |  |
|  |  | Fatal | Injury |  |  |  |  |  |  |  |  |  |
| 1 | 6/23/2012 | Sat | 3:55 PM | 0 | 0 | \$15,000 |  | End | Day | Wet | Carele | iving |  |  |  |  |
| 2 | 8/25/2012 | Sat | 11:58 AM | 0 | 2 | \$11,000 |  | Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 3 | 9/18/2012 | Tue | 12:57 AM | 0 | 1 | \$2,000 |  |  | Day | Wet | Disregarded | ol Devices |  |  |  |  |
| 4 | 11/14/2012 | Wed | 8:15 AM | 0 | 2 | \$9,500 |  |  | Day | Dry | Disregarded | ol Devices |  |  |  |  |
| 5 | 12/24/2012 | Mon | 3:52 PM | 0 | 1 | \$13,000 |  |  | Day | Dry | Failed to | ROW |  |  |  |  |
| 6 | 4/11/2013 | Thu | 7:56 AM | 0 | 2 | \$10,000 |  | Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 7 | 7/20/2013 | Sat | 4:54 AM | 0 | 3 | \$10,000 |  | End | Night | Dry | Carele | iving |  |  |  |  |
| 8 | 11/28/2013 | Thu | 2:05 PM | 0 | 0 | \$6,500 |  |  | Day | Dry | Failed to | ROW |  |  |  |  |
| 9 | 4/12/2014 | Sat | 12:52 PM | 0 | 2 | \$11,000 |  | Uurn | Day | Dry | Failed to | ROW |  |  |  |  |
| 10 | 5/12/2014 | Mon | 2:12 PM | 0 | 2 | \$8,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 11 | 7/3/2014 | Thu | 1:30 PM | 0 | 1 | \$15,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 12 | 1/10/2015 | Sat | 3:25 PM | 0 | 4 | \$10,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 13 | 9/8/2015 | Tue | 12:00 PM | 0 | 4 | \$13,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 14 | 4/25/2016 | Mon | 12:15 PM | 0 | 7 | \$12,000 |  |  | Day | Dry |  |  |  |  |  |  |
| 15 | 6/6/2016 | Mon | 11:56 AM | 0 | 1 | \$17,000 |  |  | Day | Dry | Failed to | ROW |  |  |  |  |
| 16 | 9/4/2016 | Sun | 1:40 PM | 0 | 2 | \$24,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 17 | 12/2/2016 | Fri | 3:38 PM | 0 | 1 | \$55 |  |  | Day | Dry | Carele | iving |  |  |  |  |
| 18 | 3/25/2017 | Sat | 3:41 PM | 0 | 0 | \$6,000 |  | Jurn | Day | Dry | Failed to | ROW |  |  |  |  |
| 19 | 4/2/2017 | Sun | 5:27 PM | 0 | 1 | \$17,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 20 | 5/11/2017 | Thu | 12:36 PM | 0 | 4 | \$5,000 |  |  | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 21 | 9/14/2017 | Thu | 6:30 PM | 0 | 2 | \$25,000 |  |  | Night | Dry | Failed to | ROW |  |  |  |  |
| 22 | 11/18/2017 | Sat | 6:55 AM | 0 | 3 | \$3,200 |  | Jurn | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 23 | 12/3/2017 | Sun | 12:33 AM | 0 | 0 | \$5,000 |  | End | Night | Dry |  |  |  |  |  |  |
| 24 | 4/21/2018 | Sat | 4:00 PM | 0 | 0 | \$7,000 |  |  | Day | Dry | Disregarded | ol Devices |  |  |  |  |
| TOTAL |  |  |  | 0 | 45 | \$255,255 |  |  |  |  |  |  |  |  |  |  |
| Total No. | Fatal | Injury | PDO | Rear End | Head-on | Angle | Left <br> Turn | Right Turn | Sideswipe | Off <br> Road | Bicycle / Pedestrian | Other |  |  |  |  |
| 24 | 0 | 19 | 24 | 3 | 0 | 15 | 4 | 1 | 0 | 0 | 0 | 1 |  |  |  |  |
| PERCENT | 0\% | 79\% | 100\% | 13\% | 0\% | 63\% | 17\% | 4\% | 0\% | 0\% | 0\% | 4\% |  |  |  |  |
| Contrib. Cause | Day | Night | PAVEMENT CONDITIONS |  |  | $\begin{aligned} & \text { Exceeded } \\ & \text { Speed } \end{aligned}$ | DUI | Careless Driving | Improper Lane Change | $\begin{array}{\|l} \hline \text { Failed to } \\ \text { Yield } \\ \text { ROW } \\ \hline \end{array}$ | Disregarded <br> Control <br> Devices | Other |  |  |  |  |
|  |  |  | WET | DRY | Unknown |  |  |  |  |  |  |  |  |  |  |  |
| TOTAL | 21 | 3 | 2 | 22 | 0 | 0 | 0 | 3 | 0 | 8 | 11 | 2 |  |  |  |  |
| PERCENT | 88\% | 13\% | 8\% | 92\% | 0\% | 0\% | 0\% | 13\% | 0\% | 33\% | 46\% | 8\% |  |  |  |  |
| Total Vehicles Entering/ADT: |  |  |  |  |  |  | Collision | Rate: |  |  |  | R M.E.V. |  |  |  |  |



Form 750-020-06


|  Form 750-020-06 <br> State of Florida Department of Transportation TRAFFIC ENGINEERING <br> COLLISION SUMMARY $10 / 15$ |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| General Information |  |  |  |  |  |  |  |  |  |  |  |  |
| Section/Roadway ID: <br> Intersecting Route: <br> Milepost: <br> County: |  | San Miguel Road |  |  |  | State Road: <br> Study Period: <br> Data by: <br> Date: |  |  | Marigold Avenue |  |  |  |
|  |  | 1/1/12 | To: | 3/31/19 |  |  |  |
|  |  | San Miguel Road | ETM |  |  |  |
|  |  | Osceola | 9/30/2019 |  |  |  |
| No. | Date |  |  |  |  | Day | Time | Severity |  | Property Damage | Crash Type |  | Day / Night | Wet / Dry | Contributing Cause |  |
|  |  | Fatal | Injury |  |  |  |  |  |  |  |  |  |
| 1 | 6/10/2012 | Sun | 2:55 PM | 0 | 5 | \$4,500 | Rear End |  | Day | Dry | Careless Driving |  |  |  |  |  |
| 2 | 7/7/2012 | Sat | 2:45 PM | 0 | 1 | \$14,000 |  | ar End | Day | Dry | Carel | iving |  |  |  |  |
| 3 | 7/28/2012 | Sat | 8:20 PM | 0 | 0 | \$50 |  | ar End | Night | Dry |  |  |  |  |  |  |
| 4 | 10/5/2012 | Fri | 5:25 PM | 0 | 0 | \$2,100 |  | Road | Day | Wet | Carel | riving |  |  |  |  |
| 5 | 12/31/2012 | Mon | 12:56 PM | 0 | 2 | \$13,000 |  | ther | Day | Dry | Failed to | ROW |  |  |  |  |
| 6 | 1/27/2013 | Sun | 1:20 PM | 0 | 2 | \$3,400 |  | ar End | Day | Dry | Carel | riving |  |  |  |  |
| 7 | 1/23/2014 | Thu | 8:45 PM | 0 | 1 | \$900 |  | ar End | Night | Dry | Carel | riving |  |  |  |  |
| 8 | 8/2/2014 | Sat | 5:32 PM | 0 | 1 | \$1,000 |  | Road | Day | Dry |  |  |  |  |  |  |
| 9 | 10/19/2014 | Sun | 3:25 AM | 0 | 1 | \$2,500 |  | Road | Night | Dry | Carele | riving |  |  |  |  |
| 10 | 11/9/2014 | Sun | 12:47 AM | 0 | 0 | \$11,500 |  | ther | Night | Wet | Failed to | ROW |  |  |  |  |
| 11 | 6/27/2015 | Sat | 6:15 PM | 0 | 1 | \$6,000 |  | ar End | Day | Dry | Carel | riving |  |  |  |  |
| 12 | 7/23/2016 | Sat | 1:33 PM | 0 | 3 | \$14,000 |  | eswipe | Day | Dry | Failed to | ROW |  |  |  |  |
| 13 | 8/26/2016 | Fri | 2:20 PM | 0 | 0 | \$2,100 |  | ther | Day | Dry | Improp | cking |  |  |  |  |
| 14 | 12/6/2016 | Tue | 4:10 PM | 0 | 0 | \$18,000 |  | ar End | Day | Wet | Followed | Closely |  |  |  |  |
| 15 | 1/14/2017 | Sat | 11:45 PM | 0 | 2 | \$2,300 |  | ht Turn | Night | Dry | Disregarded | rol Devices |  |  |  |  |
| 16 | 3/9/2017 | Thu | 7:56 AM | 0 | 2 | \$1,400 |  | ar End | Day | Dry | Carele | riving |  |  |  |  |
| 17 | 6/12/2017 | Mon | 8:00 PM | 0 | 1 | \$8,075 |  | ar End | Day | Wet | Carele | iving |  |  |  |  |
| 18 | 10/5/2017 | Thu | 11:01 AM | 0 | 1 | \$500 |  | llover | Day | Wet |  |  |  |  |  |  |
| 19 | 12/5/2017 | Tue | 8:42 AM | 0 | 3 | \$7,000 |  | ar End | Day | Dry | Carele | iving |  |  |  |  |
| 20 | 2/26/2018 | Mon | 9:57 PM | 0 | 1 | \$8,000 |  | ar End | Night | Dry | Carele | riving |  |  |  |  |
| 21 | 4/4/2018 | Wed | 3:35 PM | 0 | 0 | \$7,000 |  | llover | Day | Dry | Carele | riving |  |  |  |  |
| 22 | 4/17/2018 | Tue | 4:22 PM | 0 | 0 | \$550 |  | cycle | Day | Dry | Failed to | ROW |  |  |  |  |
| 23 | 9/8/2018 | Sat | 3:37 AM | 0 | 2 | \$15,000 |  | ar End | Day | Wet | Carele | iving |  |  |  |  |
| 24 | 12/29/2018 | Sat | 2:26 PM | 0 | 0 | \$3,000 |  | ar End | Day | Dry | Carele | iving |  |  |  |  |
| TOTAL |  |  |  | 0 | 29 | \$145,875 |  |  |  |  |  |  |  |  |  |  |
| Total No. | Fatal | Injury | PDO | Rear End | Head-on | Angle | Left <br> Turn | Right Turn | Sideswipe | Off <br> Road | Bicycle / Pedestrian | Other |  |  |  |  |
| 24 | 0 | 16 | 24 | 13 | 0 | 0 | 0 | 1 | 1 | 3 | 1 | 5 |  |  |  |  |
| PERCENT | 0\% | 67\% | 100\% | 54\% | 0\% | 0\% | 0\% | 4\% | 4\% | 13\% | 4\% | 21\% |  |  |  |  |
| Contrib. | Day | Night | PAVEM | ENT CONDI | TIONS | Exceeded | DUI | Careless | Improper Lane | Failed to Yield | Disregarded Control | Other |  |  |  |  |
| Cause | Day | Nigh | WET | DRY | Unknown | Speed |  | Driving |  |  |  |  |  |  |  |  |
| TOTAL | 18 | 6 | 6 | 18 | 0 | 0 | 0 | 14 | 0 | 4 | 1 | 5 |  |  |  |  |
| PERCENT | 75\% | 25\% | 25\% | 75\% | 0\% | 0\% | 0\% | 58\% | 0\% | 17\% | 4\% | 21\% |  |  |  |  |
| Total Vehic | es Entering/ |  |  |  |  |  | Collision | Rate: |  |  |  | R M.E.V. |  |  |  |  |


| State of Florida Department of Transportation Form 750-020-06 <br> COLLISION SUMMARY TRAFFIC ENGINEERING <br> $10 / 15$  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| General Information |  |  |  |  |  |  |  |  |  |  |  |  |
| Section/Roadway ID: Intersecting Route: Milepost: County: |  | San Lorenzo |  |  |  | State Road: <br> Study Period: <br> Data by: <br> Date: |  |  | Marigold Avenue |  |  |  |
|  |  | 1/1/12 | To: | 3/31/19 |  |  |  |
|  |  |  |  |  |  |  |  |  | ETM |  |  |  |
|  |  | Osceola | 6/21/2019 |  |  |  |
| No. | Date |  |  |  |  | Day | Time | Severity |  | Property Damage | Crash Type |  | Day / Night | $\begin{gathered} \text { Wet / } \\ \text { Dry } \end{gathered}$ | Contributing Cause |  |
|  |  | Fatal | Injury |  |  |  |  |  |  |  |  |  |
| 1 | 1/24/2012 | Tue | 12:40 PM | 0 | 0 | \$650 |  | Sideswipe | Day | Dry | Improp | sing |  |  |  |  |
| 2 | 4/19/2012 | Thu | 10:30 PM | 0 | 0 | \$1,000 |  | Sideswipe | Night | Dry |  |  |  |  |  |  |
| 3 | 6/1/2012 | Fri | 5:06 PM | 0 | 0 | \$10,000 |  | Left Turn | Day | Wet | Failed to | ROW |  |  |  |  |
| 4 | 6/24/2012 | Sun | 11:55 AM | 0 | 5 | \$10,000 |  | Angle | Day | Wet | Disregarde | rol Devices |  |  |  |  |
| 5 | 7/2/2012 | Mon | 10:26 PM | 0 | 2 | \$25,000 |  | Angle | Night | Dry | Failed to | ROW |  |  |  |  |
| 6 | 10/5/2012 | Fri | 6:52 PM | 0 | 0 | \$3,200 |  | Left Turn | Day | Wet | Failed to | ROW |  |  |  |  |
| 7 | 2/26/2013 | Tue | 1:03 PM | 0 | 3 | \$11,500 |  | Rear End | Day | Wet | Care | iving |  |  |  |  |
| 8 | 4/27/2013 | Sat | 11:12 AM | 0 | 2 | \$14,000 |  | Left Turn | Day | Dry | Failed | ROW |  |  |  |  |
| 9 | 6/7/2013 | Fri | 12:15 AM | 0 | 0 | \$9,000 |  | Left Turn | Night | Wet | Failed to | ROW |  |  |  |  |
| 10 | 12/11/2013 | Wed | 7:00 PM | 0 | 2 | \$12,400 |  | Angle | Night | Dry | Failed to | ROW |  |  |  |  |
| 11 | 4/12/2014 | Sat | 12:10 PM | 0 | 1 | \$18,000 |  | Angle | Day | Dry | Failed to | ROW |  |  |  |  |
| 12 | 5/3/2014 | Sat | 10:07 AM | 0 | 0 | \$12,000 |  | Angle | Day | Wet | Failed to | ROW |  |  |  |  |
| 13 | 8/3/2016 | Wed | 3:23 PM | 0 | 1 | \$15,500 |  | Off Road | Day | Dry | Carel | iving |  |  |  |  |
| 14 | 10/30/2016 | Sun | 10:40 AM | 0 | 0 | \$12,000 |  | Left Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 15 | 11/24/2016 | Thu | 6:10 AM | 0 | 2 | \$20,200 |  | Left Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 16 | 3/19/2017 | Sun | 6:30 PM | 0 | 1 | \$12,000 |  | Angle | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 17 | 3/19/2017 | Sun | 8:31 PM | 0 | 1 | \$20,000 |  | Left Turn | Night | Dry | Failed to | ROW |  |  |  |  |
| 18 | 4/14/2017 | Fri | 1:45 PM | 0 | 0 | \$5,000 |  | Left Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 19 | 6/4/2017 | Sun | 8:11 PM | 0 | 2 | \$13,000 |  | Left Turn | Night | Wet | Failed to | ROW |  |  |  |  |
| 20 | 7/31/2017 | Mon | 1:20 PM | 0 | 0 | \$4,500 |  | Off Road | Day | Wet | Carel | iving |  |  |  |  |
| 21 | 10/23/2017 | Mon | 5:25 PM | 0 | 1 | \$1,100 |  | Angle | Day | Wet | Carel | iving |  |  |  |  |
| 22 | 1/20/2018 | Sat | 12:00 PM | 0 | 0 | \$2,300 |  | Left Turn | Day | Dry | Failed to | ROW |  |  |  |  |
| 23 | 3/3/2018 | Sat | 9:50 AM | 0 | 6 | \$4,000 |  | Angle | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| 24 | 8/19/2018 | Sun | 7:34 AM | 0 | 2 | \$6,000 |  | Angle | Day | Dry | Disregarded | rol Devices |  |  |  |  |
| TOTAL |  |  |  | 0 | 31 | \$242,350 |  |  |  |  |  |  |  |  |  |  |
| Total No. | Fatal | Injury | PDO | Rear End | Head-on | Angle | Left Turn | Right <br> Turn | Sideswipe | Off <br> Road | Bicycle / Pedestrian | Other |  |  |  |  |
| 24 | 0 | 14 | 24 | 1 | 0 | 9 | 10 | 0 | 2 | 2 | 0 | 0 |  |  |  |  |
| PERCENT | 0\% | 58\% | 100\% | 4\% | 0\% | 38\% | 42\% | \% 0\% | 8\% | 8\% | 0\% | 0\% |  |  |  |  |
| Contrib. | Day | Night | PAVEM | ENT CONDI | TIONS | Exceeded | DU | Careless | Improper | Failed to | Disregarded | Other |  |  |  |  |
| Cause | D | Nigh | WET | DRY | Unknown | Speed | DUI | Driving | Change | ROW | Devices | Other |  |  |  |  |
| TOTAL | 18 | 6 | 9 | 15 | 0 | 0 | 1 | 4 | 0 | 14 | 4 | 1 |  |  |  |  |
| PERCENT | 75\% | 25\% | 38\% | 63\% | 0\% | 0\% | 4\% | 17\% | 0\% | 58\% | 17\% | 4\% |  |  |  |  |
| Total Vehic | s Entering/ |  |  |  |  |  | Collisio | ion Rate: |  |  |  | R M.E.V. |  |  |  |  |




## Appendix D:

## Collision Diagrams






## Appendix E:

## Safe Routes To School Sidewalk Improvements

## Deerwood SRTS Plan



## Deerwood Elementary School Plan



## Appendix F:

## Short-Term Concept Diagrams






## Appendix G:

## Long-Term Concept Diagrams






## Appendix H:

Improvements Cost Estimates


*Unit cost taken from FDOT Statewide Historical Cost from 02/01/2019 to 01/31/2020

## Appendix I:

## B/C Analysis, Net Present Value, and <br> Crash Modification Factor

## CMF / CRF Details

CMF ID: 5228

Conversion of intersection into low-speed roundabout
Description: Conversion of intersection into low-speed roundabout
Prior Condition: The intersection was operating under no control, yield, TWSC, AWSC, or signal control.

Category: Intersection geometry
Study: Evaluation of Roundabout Safety, Qin et al., 2013

| Crash Modification Factor (CMF) |  |
| :---: | :---: | :---: |
| Value: | 0.473 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.113 |

## Crash Reduction Factor (CRF)

Value: $\quad 52.73$ (This value indicates a decrease in crashes)

Adjusted Standard Error:

## Unadjusted Standard Error:

11.3

## Applicability

| Crash Type: | All |
| :--- | :--- | :--- |
| Crash Severity: | K (fatal),A (serious injury),B (minor injury),C (possible injury) |
| Roadway Types: | Not specified |
| Number of Lanes: | 2,4 |
| Road Division Type: | All |
| Speed Limit: |  |
| Area Type: | All |
| Traffic Volume: |  |

## If countermeasure is intersection-based

## Intersection Type: Roadway/roadway (not interchange related)

Intersection Geometry:

Traffic Control:

Major Road Traffic Volume:
4100 (total entering) to 48100 (total entering) Annual Average Daily Traffic (AADT)

Minor Road Traffic Volume:

## Development Details

Date Range of Data Used:

Municipality:

State:

| Country: | USA |
| ---: | :--- | :--- |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |
| Sample Size Used: | Crashes |
| Before Sample Size Used: | 55 Crashes |
| After Sample Size Used: | 26 Crashes |


| Included in Highway Safety |
| :---: | :--- | :--- |
| Manual? |$\quad$ No | Other Details |
| :--- |
| Date Added to Clearinghouse: |
| Aug-01-2013 |
| Comments: | | - Study included three-yearbefore and after crash datafor each site.- In |
| :--- |
| this case, the reported before-crashes represent the "expected |
| crashes" after treatment.- "Traffic Control" includes intersections with |
| yield control, two-way stop-control, all-way stop-control, and signal |
| control.- Reported traffic volume is total entering volume. |

This site is funded by the U.S. Department of Transportation Federal Highway Administration and maintained by the University of North Carolina Highway Safety Research Center

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## CMF / CRF Details

CMF ID: 7999

Install left-turn lane
Description:
Prior Condition: Intersections without left turn lanes
Category: Intersection geometry
Study: Safety Evaluation of Signal Installation With and Without Left Turn Lanes on Two Lane Roads in Rural and Suburban Areas, Srinivasan et al., 2014

```
Star Quality Rating:
```

[View score details]

| Crash Modification Factor (CMF) |  |
| :---: | :--- |
| Value: | 0.566 |
| Adjusted Standard Error: |  |
| Unadjusted Standard Error: | 0.113 |


| Crash Reduction Factor (CRF) |  |
| ---: | :--- |
| Value: | 43.4 (This value indicates a decrease in crashes) |
| Adjusted Standard Error: |  |

## Applicability

| Crash Type: | All |
| :---: | :---: |
| Crash Severity: | K (fatal), A (serious injury), B (minor injury), C (possible injury) |
| Roadway Types: | Not specified |
| Number of Lanes: | 2 |
| Road Division Type: |  |
| Speed Limit: |  |
| Area Type: | All |
| Traffic Volume: |  |
| Time of Day: | All |

If countermeasure is intersection-based

| Intersection Type: |
| ---: |
| Intersection Geometry: |
| Traffic Control: |
| Major Road Traffic Volume: |
| Minor Road Traffic Volume: |

Not specified

3-leg

Signalized

2981 to 18248 Annual Average Daily Traffic (AADT)

972 to 13880 Annual Average Daily Traffic (AADT)

Development Details

| Date Range of Data Used: | 1992 to 2012 |  |
| ---: | :--- | :--- |
| Municipality: |  |  |
| State: | NC |  |
|  |  |  |


| Country: |  |
| :---: | :---: |
| Type of Methodology Used: | Before/after using empirical Bayes or full Bayes |
| Sample Size Used: |  |
|  | Other Details |
| Included in Highway Safety Manual? | No |
| Date Added to Clearinghouse: | Nov-10-2016 |
| Comments: | The CMF was developed for both rural and suburban areas. |

This site is funded by the U.S. Department of Transportation Federal Highway Administration and maintained by the University of North Carolina Highway Safety Research Center

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## STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION SAFETY OFFICE ANNUAL BENEFIT COST ANALYSIS



| Project Name | Marioold Avenue Roundabous | Year\# | 0 |  |  |  | 4 |  | 56 | 57 | ${ }^{8}$ | $\underline{9}$ | 10 | ${ }^{11}$ | ${ }^{12}$ | ${ }^{13}$ | 14 | ${ }^{15}$ | 16 | ${ }^{17}$ | ${ }^{18}$ | 19 | 20 | ${ }^{21}$ | 22 | 23 | 24 | 25 | 26 | 27 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Current Year | 2020 | Calendar Year | 2020 | 2021 | 2022 | 2023 | 2024 | 2025 | 2026 | 2027 |  | 2029 | 230 | 2031 | 2032 | 233 | 2034 | 2035 | 236 | 2037 | 2038 | 2039 | 2040 | ${ }^{241}$ | 2042 | 2043 |  |  |  |  |
| Project Completion | 2024 | Estimated Cost | 6,157,741 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Project Life |  | Estimated Benefits |  |  |  |  | 1,059,871 | 1,059,871 | 1,059,871 | 1 1,059,871 | $1,059,871$ | 1,059,871 | 1 1,059,871 | 559, | 1,059,871 | ,059,871 | 059, | 1,059,871 | 1,059, | 059 | 1,059,871 | ,059, | 1,059,871 | 1,059,871 | $1,059,871$ | 1,059,871 |  |  |  |  |
| Proiect categor |  | Calculation |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  | ${ }_{\text {- }}^{\text {- } 157,741}$ | ${ }_{0}^{0.962}$ | 0.925 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 0.513 0 |  |  | ${ }^{0.456}$ | ${ }^{0.439}$ | ${ }^{0.422}$ | ${ }_{0}^{0.406}$ | ${ }^{0.390}$ | 0.375 0 | ${ }_{0}^{0.361}$ |  |
| Constur |  | Discounted Senefits |  | 。 | $\bigcirc$ |  | 905,982 | 871,137 | 837,631 | 805,415 | 774,437 | 744,651 | 716,011 | 688,42 | 661,992 | 636,531 | 612,049 | 588,509 | 565,874 | 544,109 | 523,182 | 503,060 | 483,711 | 465,107 | 447,218 | 430,017 | $\bigcirc$ | 0 | $\bigcirc$ | $\stackrel{\square}{\circ}$ |
|  |  | NPV | 6,647,354 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Appendix J:

## Turning Movement Volumes



| DATE: May 22, 2019 (Wednesday) |
| :---: |
| LOCATION: Marigold Av \& Peabody Rd |

CITY: Poinciana
county: Osceola County

LATITUDE: 0

## LONGITUDE: 0

| Peabody Rd |  |  |  |  | Peabody Rd |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EASTBOUND |  |  |  |  | WESTBOUND |  |  |  |  | $\begin{gathered} \text { E/W } \\ \text { TOTAL } \end{gathered}$ | GRAND tOTAL |
| L | T | R | U-turn | TOTAL | L | T | R | U-turn | TOTAL |  |  |
| 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 | 4 |
| 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 2 |
| 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 3 | 6 |
| 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 2 | 6 |
| 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 7 |
| 0 | 0 | 2 | 0 | 2 | 0 | 1 | 0 | 0 | 1 | 3 | 6 |
| 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 2 | 3 |
| 0 | 1 | 4 | 0 | 5 | 1 | 2 | 0 | 0 | 3 | 8 | 22 | | 1 | 2 |
| :---: | :---: |
| 1 | 6 |
| 0 | 2 |
| 0 | 2 |
| 2 | 12 |





| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 05:15 PM | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 |
| 05:30 PM | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 1 | 0 | 1 | 3 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 4 |
| 06:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 06:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 06:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 06:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 07:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 07:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| AM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:00 AM to 08:00 AM | 1 | 3 | 3 | 0 | 7 | 0 | 6 | 1 | 0 | 7 | 14 | 0 | 1 | 4 | 0 | 5 | 1 | 2 | 0 | 0 | 3 | 8 | 22 |
| Midday Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{array}{\|c} \text { 02:45 PM to } \\ 03: 45 \mathrm{PM} \\ \hline \end{array}$ | 3 | 0 | 4 | 0 | 7 | 0 | 3 | 1 | 0 | 4 | 11 | 1 | 0 | 2 | 0 | 3 | 2 | 0 | 0 | 0 | 2 | 5 | 16 |
| PM Peak |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 05:30 PM to <br> $06: 30 \mathrm{PM}$ | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |





DATE: August 27, 2019 (Tuesday) location: Marigold Ave \& Laurel Ave

CITY: Poinciana
county: Osceola County

LATITUDE: 0 LONGITUDE: 0




DATE: August 27, 2019 (Tuesday)
LOCATION: Marigold Ave \& San Miguel Rd

CITY: Poinciana county: Osceola County

LATITUDE: 0 LONGITUDE: 0










[^0]:    ** only thirty minutes of data was reviewed

