

OSCEOLA COUNTY, FLORIDA AND INCORPORATED AREAS

Community Name	Community Number
KISSIMMEE, CITY OF	120190
OSCEOLA COUNTY	120189
(UNINCORPORATED AREAS)	
REEDY CREEK IMPROVEMENT	120577
DISTRICT	
ST. CLOUD, CITY OF	120191



REVISED June 18, 2013



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER 12097CV000A

NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this Flood Insurance Study may be revised and republished at any time. In addition, part of this Flood Insurance Study may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the Flood Insurance Study. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current Flood Insurance Study components.

Initial Countywide FIS Effective Date: May 7, 2001

First Revised Countywide FIS Date: June 18, 2013

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FLOOD INSURANCE STUDY OSCEOLA COUNTY, FLORIDA, AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This Flood Insurance Study (FIS) revises and updates information on the existence and severity of flood hazards in the geographic area of Osceola County, including the Cities of Kissimmee and St. Cloud; Reedy Creek Improvement District; and the unincorporated areas of Osceola County (referred to collectively herein as Osceola County), and aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This study has developed flood-risk data for various areas of the community that will be used to establish actuarial flood insurance rates and to assist the community in its efforts to promote sound floodplain management. Minimum floodplain management requirements for participation in the National Flood Insurance Program (NFIP) are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

The Reedy Creek Improvement District is located in more than one county; the flood hazard information for the portion of this community located in Orange County is included in the FIS report for Orange County, Florida, and Incorporated Areas (Reference 1).

In some states or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence, and the State (or other jurisdictional agency) will be able to explain them.

The Flood Insurance Rate Map (FIRM) and FIS report for this countywide study have been produced in digital format. Flood hazard information was converted to meet the Federal Emergency Management Agency (FEMA) FIRM database specifications and geographic information standards and is provided in a digital format so that it can be incorporated into a local Geographic Information System and be accessed more easily by the community.

1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

For this revision of the countywide FIS, new hydrologic and hydraulic analyses were prepared by BakerAECOM, LLC, for FEMA, under Contract No. HSFEHQ-09-D-0368, Task R4-TO66. This revised study was completed in March 2011.

For the initial countywide FIS, the hydrologic and hydraulic analyses were prepared by Engineering Methods & Applications, Inc., for FEMA, under Inter-Agency Agreement No. EMW-95-C-4705. That work was completed in July 1996.

The initial countywide FIS was prepared to include all jurisdictions within Osceola County in a countywide FIS. Information on the authority and acknowledgements for each jurisdiction with a previously printed FIS report included in the countywide FIS is shown below:

Kissimmee, City of:

The hydrologic and hydraulic analyses for the FIS report dated January 2, 1981, were prepared by the U.S. Army Corps of Engineers (USACE), Jacksonville District, for the Federal Insurance Administration (FIA), under Inter-Agency Agreement No. IAA-H-1878, Project Order No. 10. That work was completed in June 1979.

Osceola County (Unincorporated areas):

The hydraulic analyses for the FIS report dated August 3, 1981, were prepared by the USACE, Jacksonville District, for FEMA, under Inter-Agency Agreement No. IAA-H-1878, Project Order No. 10, Amendment No. 1. That work was completed in January 1980.

The hydrologic and hydraulic analyses for the FIS report dated March 16, 1989, were prepared by the USACE, Jacksonville District for FEMA, under Inter-Agency Agreement No. IAA-H-1878, Project Order No. 10, Amendment No. 1. That work was completed in January 1980.

The hydrologic and hydraulic analyses for the FIS report dated November 20, 1996, were prepared by Post, Buckley, Schuh & Jernigan, Inc.

St. Cloud, City of:

The hydrologic and hydraulic analyses for the FIS report dated March 1980 were prepared by the USACE, Jacksonville District, for the FIA, under Inter-Agency Agreement No. IAA-H-1878, Project Order No. 10. That work was completed in February 1979.

For the FIS report dated April 3, 1996, flooding information was taken from the FIS for the unincorporated areas of Osceola County because of corporate limits changes and flooding mismatches between the City of St. Cloud and the unincorporated areas of Osceola County (Reference 2).

The authority and acknowledgments for Reedy Creek Improvement District are not included because there was no previously printed FIS report for this community.

Base map information shown on the FIRM was provided in digital format by Osceola County Planning Office.

The coordinate system used for producing the FIRM is the Florida State Plane FIPS 0901. Corner coordinates shown on the FIRMs are in latitude and longitude referenced to the UTM projection, North American Datum (NAD 83) HARN and the GRS80. Distance units were measured in feet.

1.3 Coordination

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An initial Consultation Coordination Officer (CCO) meeting (also occasionally referred to as the Scoping meeting) is held with representatives of the communities, FEMA, and the study contractors to explain the nature and purpose of the FIS and to identify the streams to be studied by detailed methods. A final CCO (often referred to as the Preliminary DFIRM Community Coordination, or PDCC, meeting) is held with representatives of the communities, FEMA, and the study contractors to review the results of the study.

For this revision of the countywide FIS, the initial CCO meeting was held on November 3, 2009, and attended by community officials, representatives of the St. Johns River and South Florida Water Management Districts, the State of Florida, FEMA Region IV, and the study contractor, Baker AECOM, LLC.

The final CCO meeting was held on August 16, 2011 to review and accept the results of this FIS. Those who attended this meeting included representatives of St. Cloud, Kissimmee, Osceola County, AECOM, and FEMA. All problems raised at that meeting have been addressed in this study.

The dates of the historical initial and final CCO meetings held for the communities within the boundaries of Osceola County are shown in Table 1, "Historical CCO Meeting Dates."

Table 1: Historical CCO Meeting Dates

Community Name	Initial CCO Date	Final CCO Date
Kissimmee, City of	December 13, 1977	March 13, 1980
Osceola County and Incorporated Areas (countywide)	September 22, 1994	September 29, 1998
Osceola County (Unincorporated Areas)	February 23, 1978	March 2, 1981
St. Cloud, City of	December 13, 1977	July 10, 1979

2.0 AREA STUDIED

2.1 Scope of Study

This FIS report covers the geographic area of Osceola County, Florida, including the incorporated communities listed in Section 1.1. The scope and methods of this study were proposed to, and agreed upon, by FEMA and Osceola County.

For this revision, a total of 9.8 additional stream miles were studied using detailed methods. Floodplain boundaries of streams that had been previously studied by detailed methods were redelineated based on more detailed and up-to-date topographic mapping for this FIS report.

All or portions of the flooding sources in Table 2, "Flooding Sources Studied by Detailed Methods," were studied by detailed methods. Limits of detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2). The areas studied by detailed methods were selected with priority given to all known flood hazards and areas of projected development or proposed construction.

Table 2: Flooding Sources Studied by Detailed Methods

Flooding Source	Reach Length (miles) or Area (square miles)	Limits of Study
Alligator Canal	1.3	2
Alligator Lake	16.4	For its entire shoreline within Osceola County
Bass Slough ¹	5.5	From mouth to a point approximately 1,260 feet upstream of Birchwood Circle
Bass Slough Tributary ¹	0.4	From the confluence with Bass Slough to a point approximately 2,040 feet upstream
Boggy Creek	1.6	2
Brick Lake	4.3	For its entire shoreline within Osceola County
Brown Lake	0.1 sq. mi.	2
C-33 Canal	2.4	From its confluence with Alligator Lake to its confluence with Lake Gentry
Canal 32-C	2.0	2
Canoe Creek (C-34 Canal)	3.5	From Canoe Creek Road to Lake Gentry

Table 2: Flooding Sources Studied by Detailed Methods (continued)

Flooding Source		Limits of Study
Coon Lake	1.8	For its entire shoreline within Osceola County
Cox Creek	1.7	2
Cypress Lake	0.01 sq. mi.	2
Davenport Creek	7.5	From its confluence with Reedy Creek to Oak Island Drive
Davenport Creek Tributary No. 1	1.0	From its confluence with Davenport Creek to Oak Island Drive
Davenport Creek Tributary No. 2	1.6	From its confluence with Davenport Creek to a point approximately 0.86 mile upstream of confluence
Dead River	0.5	2
East City Canal	3.2	From its confluence with Lake Tohopekaliga to just downstream of Oak Street
East City Canal Tributary 11	0.4	From the confluence with East City Canal to a point approximately 2,370 feet upstream
East Lake Tohopekaliga	20.1	2
Gator Bay Branch	1.1	2
Heart Lake	0.01 sq. mi.	2
Jackson Canal	1.7	2
Jim Branch	0.6	2
Kissimmee River	3.0	2
Lake Bullock	1.4	
Lake Cecil	1.6	2
Lake Center	3.3	For its entire shoreline within Osceola County
Lake Davenport	1.0	For its entire shoreline within Osceola County
Lake Gentry	0.7	For its entire shoreline within Osceola County

Table 2: Flooding Sources Studied by Detailed Methods (continued)

Flooding Source	Reach Length (miles) or Area (square miles)	Limits of Study
Lake Hatchineha	21.8	For its entire shoreline within Osceola County
Lake Joel	2.5	For its entire shoreline within Osceola County
Lake Kissimmee	48.2	2
Lake Lizzie	4.9	For its entire shoreline within Osceola County
Lake Marian ¹	8.3	2
Lake Martha	0.01 sq. mi.	2
Lake Myrtle	4.3	For its entire shoreline within Osceola County
Lake Poinsett	3.1	2
Lake Preston	5.4	For its entire shoreline within Osceola County
Lake Runnymede	3.3	2
Lake Russell	4.6	2
Lake Tohopekaliga	39.2	2
Lake Wilson	1.5	2
Live Oak Lake	3.1	2
Lonesome Camp Swamp	0.7	2
Mill Slough	4.1	2
NP (unnamed streams)	117.6	2
Orange Hammock Slough	1.4	2
Orchid Creek	0.7	2
Paradise Island	0.1 sq. mi.	2
Pearl Lake	1.5	2
Peg Horn Slough	3.2	From a point approximately 150 feet upstream of its confluence with St. Cloud Canal (Canal 31) to a point approximately 950 feet upstream of Missouri Avenue
Pennywash Creek	0.7	2

Table 2: Flooding Sources Studied by Detailed Methods (continued)

Flooding Source	Reach Length (miles) or Area (square miles)	Limits of Study
Reedy Creek	18.1	2
Reedy Creek Tributary No. 1	1.9	From its confluence with Reedy Creek to a point approximately 0.47 mile upstream of San Miguel Road
Reedy Creek Tributary No. 2	1.3	From its confluence with Reedy Creek to a point approximately 900 feet upstream of Marigold Avenue
Reedy Creek Tributary No. 3	2.8	From its confluence with Reedy Creek to San Remo Road
Rocky Pond	0.6 sq. mi.	2
Sardine Lake	1.4	2
Scrub Slough	1.6	2
Shingle Creek	9.2	From its confluence with Lake Tohopekaliga to a point approximately 100 feet upstream of Osceola Parkway
Short Canal	2.7	2
South Port Canal	0.4	2 .
St. Cloud Canal (Canal 31)	1.9	2
St. Johns River	20.2	2
Tributary No. 1	0.03	2
Trout Lake	3.1	For its entire shoreline within Osceola County
West Branch Shingle Creek ⁴	6.7	From its confluence with Shingle Creek to a point approximately 4,200 feet upstream of Scott Boulevard
West City Canal	2.3	From its confluence with Lake Tohopekaliga to just upstream of Oak Street
WPA Canal	5.2	From just upstream of Old Canoe Creek Road to St. Cloud Airfield
WPA Canal Tributary 1 ¹	2.7	From the confluence with WPA Canal to a point approximately 2.7 miles upstream

Table 2: Flooding Sources Studied by Detailed Methods (continued)

Flooding Source	Reach Length (miles) or Area (square miles)	Limits of Study
WPA Canal Tributary 1-11	0.8	From the confluence with WPA Canal Tributary 1 to a point approximately 4,000 feet upstream
Other Lakes/Ponds ³	151.9 sq. mi.	2

¹ Flooding source with new or revised analyses incorporated as part of the current study update

All or portions of numerous streams were studied by approximate methods in previous studies; most of these studies were refined and Zone A was re-established in this revision. The flooding sources studied by approximate methods are listed in Table 3. Approximate analyses were used to study those areas having a low development potential or minimal flood hazards.

Table 3: Flooding Sources Studied by Approximate Methods

Flooding Source	Reach Length (miles) or Area (square miles)
Bass Slough	0.5
Beef Camp Branch	1.3
Blanket Bay Slough	7.7
Blue Cypress Creek	18.8
Boggy Branch	2.1
Bonnet Creek ¹	1.1
Bonnet Gully	2.6
Buck Lake	0.8 sq. mi.
Bull Creek	19.2
Cabbage Slough	2.4
Cat Lake	3.2 sq. mi.
Cow Log Branch	6.1
Cox Creek	6.4

² Data not available

³Lakes and ponds were calculated from the Osceola County, Fl, GIS waterbodies datalayer

⁴In this revision, West Branch Shingle Creek Tributary has been combined with and described under West Branch Shingle Creek (per LOMR 08-04-1601P)

Table 3: Flooding Sources Studied by Approximate Methods (continued)

Flooding Source	Reach Length (miles) or Area (square miles)
Crabgrass Creek	10.7
Cypress Lake	0.1 sq. mi.
Davenport Creek	0.9
Dead River	3.6
Elbow Branch	3.1
Fish Lake	0.4 sq. mi.
Gap Creek	2.9
Garrett Branch	3.5
Gator Branch	6.6
Hammock Branch	1.9
Hatchineha Canal	2.4
Hog Pen Slough	0.3 sq. mi.
Indian Branch	2.7
Jackson Canal	4.5
Jane Green Creek	3.6
Jim Branch	0.8
Kissimmee River	12.9
Lake Jackson	3.2
Lake Marian	8.0
Little Creek	3.2
Little North Prong	1.9
Major Sloush	0.2 sq. mi.
Mill Slough	0.9
North Branch Crabgrass Creek	2.2
North Fork Taylor Creek	5.2
NP (unnamed streams)	916.0
NP - Priority 1- Poinciana	0.6
NP - Priority 4 – Kennansville	14.7
Orchid Creek	7.5
Padgett Branch	0.8

Table 3: Flooding Sources Studied by Approximate Methods (continued)

Flooding Source	Reach Length (miles) or Area (square miles)
Pagett Branch	3.5
Peg Horn Slough	0.4
Pennywash Creek	9.0
Pine Island Slough	13.7
Rauslerson Branch	3.8
Reedy Creek	8.1
Rocky Pond	1.9 sq. mi.
Scrub Slough	2.6
Short Canal	2.5
Sixmile Creek	1.7
South Fork Taylor Creek	6.8
South Port Canal	5.0
South Prong Alafia River	2.1
Taylor Creek	2.1
Tenmile Creek	5.7
Tracy Branch	5.4
Tyson Creek	5.7
West Branch Crabgrass Creek	7.9
Wolf Creek	2.8
WPA Canal1	0.1
Yoke Branch	3.3
Other Lakes/Ponds ²	41.6 sq. mi.

¹Not restudied; data taken from previous countywide FIS report

This revision to the countywide FIS also incorporates the determination of letters issued by FEMA resulting in Letters of Map change as shown in Table 4, "Letters of Map Revision (LOMRs) Incorporated into Current Study." Note that LOMR Case Number 08-04-1601P combined West Branch Shingle Creek Tributary and West Branch Shingle Creek, so all other tables in this revision to the countywide FIS do not list West Branch Shingle Creek Tributary separately from West Branch Shingle Creek.

²Lakes and ponds were calculated from the Osceola County, Fl, GIS waterbodies datalayer

Table 4: Letters of Map Revision (LOMRs)
Incorporated into Current Study

Case Number	Flooding Source(s)	Communities Affected	Effective Date
01-04-537P	Buck Lake	Osceola County, Florida (Unincorporated Areas)	12/07/2001
03-04-309P	Tributary No. 1	Osceola County, Florida (Unincorporated Areas)	12/22/2003
08-04-1601P	West Branch Shingle Creek West Branch Shingle Creek Tributary	Osceola County, Florida (Unincorporated Areas) City of Kissimmee	8/24/2009
09-04-6066P	WPA Canal	Osceola County, Florida (Unincorporated Areas)	7/30/2010
11-04-0259P	Unnamed Flooding Area	Osceola County, Florida (Unincorporated Areas)	3/22/2011

2.2 Community Description

Osceola County is located in central Florida. The county has an approximate total land area of 1,506 square miles. Osceola County is bordered by Orange County to the north, Brevard County to the northeast, Indian River County to the southeast, Okeechobee County to the south, and Polk County to the west. The population of Osceola County was estimated to be 270,618 in 2009 (Reference 3).

The floodplains of Osceola County consist of lowlands adjacent to the streams and lakes. The topography of Osceola County is relatively flat with some gently rolling hills. Ground elevations in Osceola County range from less than 5 feet NAVD 88 to 190 feet NAVD 88.

Osceola County's climate is semitropical, characterized by warm, humid summers, and mild dry winters. Daily maximum temperatures average 90 degrees Fahrenheit (°F) in summer, and average daily minimums are approximately 50°F in the winter. Temperature extremes of over 100°F or under 20°F are rare. The mean annual temperature is 72°F. The average annual precipitation over the study area is approximately 52 inches, and approximately 72 percent occurs from May to October, the rainy season.

Osceola County's soils consist of 11 soil associations. Three of these are either moderately well drained or excessively drained. The rest are soils that vary between poorly drained to very poorly drained (Reference 4).

In 2009 accommodation and food services was the larges economic sector, but agriculture makes up 63% of the land use. Cattle, citrus and commercial sod are the major agricultural products. The State of Florida has purchased 102,500 acres for land preservation; most of this acreage is included in the wildlife management areas Bull Creek, Triple N Ranch, and Three Lakes. The Nature Conservancy also maintains 4,730 acres (Reference 5).

2.3 Principal Flood Problems

Floods can occur in Osceola County at any time during the year; however, they are most frequent from June to October. Floods on the lakes can result from prolonged heavy rainfall over the study area combined with high antecedent lake stages. Floods on the streams can result from prolonged heavy rainfall over a large area. The flooding is usually more severe from rainfall associated with hurricanes or tropical storms and when antecedent rainfall has resulted in saturated ground conditions, which causes infiltration to be minimal. Cloudburst storms can occur at any time, but do not constitute a serious flood hazard in the study area.

Because Osceola County was practically undeveloped before the 1900s, flood records are scarce. The flood history, however, has been quite similar to that of the adjacent upper St. John's River basin, where records show that floods much greater than any of recent record occurred in 1871, and 1898, and that the floods of 1910, 1914, 1924, and 1933 were probably comparable with more recent major occurrences. In recent years, floods causing significant damage in Osceola County occurred in 1945, 1947, 1948, 1949, 1953, 1956, 1958, 1960, and 1968. Among these, the 1953 and 1960 floods were the highest of record.

The stages produced by the 1953 flood are generally the most severe of the floods on record. The flood-producing rainfall was one of the heaviest ever recorded in the study area, approximately 47 inches from June through October. Damages were severe throughout the study area and especially large for the cattle and food crop industries.

The flood of 1960 occurred when above normal seasonal rainfall coincided with the heavy rains of Hurricane Donna and Tropical Storm Florence in September. Discharges on the Kissimmee River below Lake Kissimmee peaked at 6,830 cubic feet per second (cfs) on September 30. This was near the maximum recorded discharge of 8,820 cfs which occurred in 1948.

In December 1997, heavy rains following a month of wet weather caused flash flooding of seventeen mobile homes and caused \$400,000 in damages in St. Cloud. In October 2005, flooding damages occurred in St. Cloud when Hurricane Wilma resulted in 10 inches of rain across north and northwest Osceola County (Reference 6).

Slow moving thunderstorms over Osceola County on July 11, 2009, produced 4-5 inches of rain in less than 2 hours, resulting in accumulation of deep water northwest of Kissimmee. One hundred and nine homes were flooded, with 16 receiving major damage. Property damage as the result of this flooding was estimated at \$4 million (Reference 6).

2.4 Flood Protection Measures

The USACE has constructed numerous flood protection measures in Osceola County. Regulation of the Kissimmee Lakes took place over a six-year period, from 1964 to 1970. It began with East Lake Tohopekaliga and proceeded down the west chain of lakes to Lake Kissimmee. Regulation of the west chain of lakes through Lake Kissimmee was essentially complete by 1965. Work then proceeded up the east chain of lakes reaching Lake Alligator by 1967. The regulation of the portion of the east chain of lakes from

Alligator to East Lake Tohopekaliga was accomplished from September 1967 to May 1970. Between 1964 to 1970 interim regulation schedules were adopted as soon as lake outlet works were completed. The plan was to implement permanent schedules upon completion of all works, including works on the Kissimmee River (C-38). However, even though numerous efforts were made, all parties concerned could not reach a unanimous decision, and each effort ended by recommending or awaiting additional studies. In 1975, after a comprehensive hydrologic study of Kissimmee basin, and two public meetings, the Central and Southern Florida Flood Control District prepared a report entitled "Report to the Governing Board on Regulatory Levels for Lakes in the Upper Kissimmee Basin." The schedules were approved by the USACE and adopted on February 9, 1976. Because of the possibility of further revision to Kissimmee Lakes regulation, the 1976 schedules have been adopted as interim schedules.

East Lake Tohopekaliga and a small tributary lake named Ajay are regulated by S-59, located in the St. Cloud Canal (C-31) between East Lake Tohopekaliga and Lake Tohopekaliga. Regulation did not begin until 1964, although construction of S-59 and C-31 was completed in April 1963. This was principally because regulation was contingent upon Lake Tohopekaliga outlet works, which were not completed in April 1963. The original regulation schedule was modified in 1967 and was used until 1976, when the existing schedule was introduced. The levels are regulated between 53.5 to 57.0 feet NAVD 88.

Lake Tohopekaliga is regulated by 5-61, located in Southport Canal (C-35) at the south shore of the lake. Construction of S-61 and C-35 was completed in October 1963 and regulation of Lake Tohopekaliga began in early 1964. In 1971 and again in 1979 the regulation schedules for Lakes Tohopekaliga, East Tohopekaliga, Kissimmee, Cypress, and Hatchineha were modified for implementation of the draw down of water levels in Lake Tohopekaliga. The drawdowns were conducted in cooperation with the Florida Game and Fresh Water Fish Commission for enhancement of fish habitat in the lake. The levels are regulated between 50.5 to 54.0 feet NAVD 88.

Lakes Kissimmee, Hatchineha, and Cypress are regulated by a final structure, S-65, located at the outlet of Lake Kissimmee at the head of C-38. The original General Design Memorandum plan called for Lakes Hatchineha and Bypass to be regulated together, but independently of Lake Kissimmee by a structure in the canal connecting Lakes Hatchineha and Kissimmee. Levels of Hatchineha and Cypress were to be one foot higher than those for Lake Kissimmee. However, it was determined feasible to combine the regulation whereby the Kissimmee schedule would be raised 0.5 foot and Hatchineha and Cypress would be lowered 0.4 foot. Regulation began after the completion of S-65 in August 1964. The levels are regulated between 47.3 to 51.3 feet NAVD 88.

<u>Lake Gentry</u> is regulated by S-63 located in C-34 at the south end of the lake. Water levels in C-34, downstream of S-63, are further stepped down by S-63A before it discharges into Lake Cypress. Regulation began in May 1967 with completion of S-63, S-63A, and C-34. The levels are regulated between 58.0 to 60.5 feet NAVD 88.

<u>Lakes Alligator, Center, Coon, Trout, Lizzie, and Brick</u> are controlled by two structures, S-58 located in C-32 which connects Lakes Trout and Joel, and S-60 located in C-23 between Lakes Alligator and Gentry. The southern outlet S-60 and C-33 were completed

in December 1966, thereby providing some regulatory control. However, it was not until the northern outlet (S-58) was completed in October 1969 that full control was possible. The levels are regulated between 60.4 to 62.9 feet NAVD 88.

<u>Lakes Joel, Myrtle, and Preston</u> are regulated by a single structure, S-57 located in C-30 connecting Lakes Myrtle and Mary Jane. Regulation began in September 1969 upon completion of S-57 and C-30. The levels are regulated between 58.4 to 60.9 feet NAVD 88.

<u>Lakes Hart and Mary Jane</u> are regulated by a single structure, S-62 located in C-29 which discharges into Lake Ajay. Regulation began in May 1970 after completion of S-62 and C-29 in October 1969. The levels are regulated between 58.4 to 60.0 NAVD 88.

The South Florida Water Management District has proposed a management plan for the Shingle Creek basin. The plan is the result of the combined efforts of the South Florida Water Management District, the Cities of Orlando and Kissimmee, and the Counties of Range and Osceola. The key elements of the plan are:

- 1. A floodplain management program through zoning or other appropriate means.
- 2. A capital improvement program that provides for the following structural improvements to alleviate existing flooding problems:
 - a. Excavation of the channel north of the turnpike in Orange County to provide greater runoff relief during the 1-percent-annual-chance flood.
 - b. Improvements to bridges at McLeaod Road, Orlando-Vineland Road, Interstate Route 4, and Americana Boulevard in Orange County and State Routes 600 and 531 in Osceola County.
 - c. Installation of a 2,700 cfs water control structure south of Oak Ridge Road in Orange County to prevent overdrainage and control erosion.
 - d. Creation of a floodway bypass of Shingle Creek between Old Tampa Highway and U.S. 17-92, in Osceola County, to reduce flood stages during major storm events and specifically the 1-percent-annual-chance flood in the lower reaches.

FEMA specifies that all levees must have a minimum of 3 foot freeboard against 1-percent-annual-chance flooding to be considered a safe flood protection structure.

Levees exist in the study area that provide the county with some degree of protection against flooding. However, it has been ascertained that these levees do not protect the county from rare events such as the 1-percent-annual-chance flood. The criteria used to evaluate protection against the 1-percent-annual-chance flood are 1) adequate design, including freeboard, 2) structural stability, and 3) proper operation and maintenance. Levees that do not protect against the 1-percent-annual-chance flood are not considered in the hydraulic analysis of the 1-percent-annual-chance floodplain.

As of this revised FIS report, there are no certified levees per Reg. 44 CFR 65.10 in Osceola County.

3.0 ENGINEERING METHODS

For the flooding sources studied by detailed methods in the community, standard hydrologic and hydraulic study methods were used to determine the flood-hazard data required for this study. Flood events of a magnitude that is expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

For this countywide study, hydrologic analyses were carried out to establish peak discharge frequency relationships for each flooding source studied by detailed and approximate methods affecting the community. A summary of peak discharge-drainage area relationships for streams studied by detailed methods is shown in Table 5, "Summary of Discharges."

The stillwater elevations for the 10-, 2-, 1-, and 0.2-percent-annual-chance floods for all lacustrine flooding sources have been determined and are summarized in Table 6, "Summary of Stillwater Elevations." All locations are for the entire shoreline within Osceola County.

3.1.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

Peak flood discharges for 10-, 2-, 1-, and 0.2-percent-annual-chance recurrence intervals were developed for all streams studied by detailed methods (Reference 7). Peak discharges were developed for only the 1-percent-annual-chance recurrence interval for streams studied by approximate methods. The USGS regression equations for natural basins in Florida described in USGS Water-Resources Investigations Report (WRIR) 82-4012, Technique for Estimating Magnitude and Frequency of Floods on Natural-Flow Streams in Florida, was used for Bass Slough (Lower Reach), Bass Slough (Upper Reach), Bass Slough Tributary, WPA Canal Tributary 1, WPA Canal Tributary 1-1 and all approximately study streams (Reference 8). The USGS nationwide urban regression equations described in USGS Water-Supply Paper 2207, Flood

Characteristics of Urban Watersheds in the United States, were used for East City Canal Tributary 1 (Reference 9).

The basin delineations and drainage areas were determined using a digital terrain model (DTM) based on a 10' x 10' grid size digital elevation model (DEM) generated from a 1/3 arc-second National Elevation Dataset DEM, as well as 2006 LiDAR supplied by the City of Kissimmee.

3.1.2 Methods for Flooding Sources Incorporated from Previous Studies

This section describes the methodology used in previous studies of flooding sources incorporated into this FIS that were not revised for this countywide study. Hydrologic analyses were carried out to establish peak discharge-frequency relationships for each flooding source studied by detailed methods affecting the community.

Precountywide Analyses

Osceola County (Unincorporated Areas)

No long-term stream gages are located along Boggy Creek, Reedy Creek, Mill Slough, and the St. Cloud Canal in the unincorporated areas of Osceola County; the hydrologic data for these flooding sources were obtained from other studies (References 10-14).

Rainfall-frequency estimates were obtained by statistical analysis of records from eight long-term rainfall stations in and near Orange and Seminole Counties, Florida (Reference 15). Results of the rainfall study are summarized below.

Rainfal	l (inch	es)

Duration	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
24-hours	6.6	9.7	11.3	16.4
2 days	7.1	10.0	11.7	16.8
3 days	7.7	10.6	12.3	17.3
4 days	8.2	11.2	12.9	17.8
5 days	8.7	11.8	13.5	18.3
7 days	9.6	12.9	14.1	19.3
30 days	18.2	23.0	25.1	28.4

The amount of rainfall that will run off (rainfall excess) from a particular basin is less than the rainfall due to soil permeability, vegetation cover, and other characteristics. To estimate the rainfall excess, the Soil Conservation Service (SCS, now the Natural Resources Conservation Service) has developed runoff curve numbers that relate rainfall to direct runoff (Reference 16). The runoff curve numbers were used to calculate the infiltration losses based on the soil type and land use.

The volumetric-runoff method was used to analyze Brown and Buck Lakes, and Lakes Cecile, Davenport, Runnymede, and Wilson. This method involved analyzing several storms with respect to various lake basins to determine the percentage of rainfall that can be expected to run off the land and into the lakes, determining the rainfall recess, and determining the infiltration losses (Reference 16). The next step involved applying the 50-day rainfall, calculating the volume of rain falling directly on the lake by multiplying the rainfall depth by the lake areas, and calculating the volume of rainfall runoff from the land area in to the lake by determining the rainfall excess (Reference 15). The sum of the volume of rain falling on the lake and the volume of rain running off the land gives the total volume of rainfall the lake will receive during the particular storm. Area-capacity curves were developed for the lakes. The computed rainfall volumes were applied to the area-capacity curves revealing lake stage and inundated area for each particular storm.

Elevations for floods of the selected recurrence intervals on Lake Russell were determined from the hydrologic analyses for Reedy Creek (Reference 12).

Elevations for floods of the selected recurrence intervals of Lake Tohopekaliga, East Lake Tohopekaliga, and St. Cloud Canal were determined from flood routings conducted by the USACE, Jacksonville District, in conjunction with the flood control project (Reference 14). Elevations in Lake Runnymede are controlled by East Lake Tohopekaliga. Elevations in the St. Cloud Canal are controlled by Lake Tohopekaliga. In addition, there are flooding effects from Lake Mary Jane and West City Canal present in the county.

Cities of Kissimmee and St. Cloud

Rainfall-frequency estimates for Dakota Avenue Canal and Mill Slough in the City of St. Cloud and the City of Kissimmee, respectively, were obtained from the U.S. Weather Bureau Technical Paper No. 40 for the 10-, 2-, and 1-percent-annual-chance frequencies (Reference 17). The rainfall estimate for the 0.2-percent-annual-chance frequency was determined by graphical extrapolation of the rainfall-frequency curve established from Technical Paper No. 40 (Reference 17). The 24-hour rainfall-frequency estimates are as follows:

	/· · ·
Rainfall	(inches)

Duration	10-percent-	2-percent-	1-percent-	0.2-percent-	
	annual-chance	annual-chance	annual-chance	annual-chance	
24-hours	7.5	9.4	10.4	12.9	

The aforementioned SCS runoff curve numbers were used to calculate the infiltration losses based on the soil type and land use (Reference 16).

No stream gages are located on Dakota Avenue Canal and Mill Slough, which are both studied in detail in the City of St. Cloud and the City of Kissimmee,

respectively. The hydrology for these streams for the selected recurrence intervals was achieved using the SCS methodology to determine the unit hydrographs (Reference 16). Flood hydrographs were developed by applying the 24-hour rainfall excess to the unit hydrographs using the SCS Type-II storm distribution (Reference 16). The flood hydrographs were then routed and combined using the HEC-1 Flood Hydrograph package (Reference 18). The previously studied length of Dakota Avenue Canal was removed from this revision of the countywide FIS as the canal was replaced with an underground drainage system.

Elevations for floods of the selected recurrence intervals of Lake Tohopekaliga in the City of Kissimmee were determined from flood routings conducted by the USACE, Jacksonville District, in conjunction with the flood control project (Reference 14).

Revised Analyses for Countywide FIS

The hydrologic analyses used to estimate peak discharges for the St. Johns River were obtained from the St. Johns River Water Management District's report, *The Mean Annual*, 10-Year, 25-Year, and 1-percent-annual-chance Flood Profiles for the Upper St. Johns River Under Existing Conditions (Reference 19).

Hydrologic analyses for the other flooding sources were developed using the HEC-1 computer program (Reference 18). Rainfall values were determined from analysis of rain gage data for locations in and around Osceola County. Data was provided by the South Florida Water Management District and the National Weather Service (References 21 and 22). A 4-day storm was used based on historical patterns.

Times of concentration were determined using either the SCS velocity method or the SCS lag equation (Reference 23). Rainfall infiltration calculations were based on SCS curve number methods. Curve numbers were calculated based on SCS Osceola County soil map land use as determined from 1990 Florida Department of Transportation aerials and site visits (References 24 and 4).

Snyder Unit hydrographs were used to determined basin runoff. Peak rate factors were determined based on slopes of the basin and corresponding SCS peak rate values, and calibration.

Table 5: Summary of Discharges

				Peak Discharge (Cubic Feet per Second)			
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance		
BASS SLOUGH							
At mouth	4.9	544	971	1,184	1,794		
Approximately 1,800 feet upstream of Neptune Road	4.0	486	870	1,061	1,609		
Just downstream of Bill Beck Boulevard	3.1	378	682	834	1,281		
Just downstream of Boggy Creek Road	2.1	256	469	577	905		
Approximately 1.1 miles upstream of Boggy Creek Road	1.5	236	430	527	815		
Approximately 3,700 feet downstream of Birchwood Circle	0.8	126	235	291	466		
BASS SLOUGH TRIBUTARY							
At confluence with Bass Slough	0.6	76	145	181	303		
BOGGY CREEK							
At Swamp Outflow	55.7	3,000	5,090	6,310	8,920		
C-33 CANAL ¹							
At outlet for Alligator Lake	15.4	8,758	12,112	13,617	16,950		
CANOE CREEK ¹ (C-34 CANAL)							
At outlet for Lake Gentry	22.52	5,592	7,761	8,730	10,796		

¹Peak discharges computed with UNET (Reference 25)

Table 5: Summary of Discharges (continued)

	Peak Discharge (Cubic Feet per Second)				nd)
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
DAVENPORT CREEK	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			., .,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
At mouth	27.13	2,126	3,396	3,991	5,320
Approximately 0.6 mile downstream of State Route 545	26.73	1,932	3,099	3,648	4,866
At State Route 545	25.56	1,516	2,524	2,986	4,066
Approximately 0.4 mile upstream of State Route 545	25.28	1,496	2,511	2,970	4,042
Approximately 0.3 mile downstream of Interstate Route 4	24.94	1,487	2,491	2,944	4,066
Approximately 0.6 mile downstream of Keefer Trail	22.49	1,417	2,368	2,798	3,813
At Keefer Trail	22.20	1,416	2,358	2,785	3,791
Approximately 0.6 mile upstream of Keefer Trail	14.88	363	628	820	1,346
At confluence of Davenport Creek Tributary No. 2	8.53	898	1,466	1,718	2,341
Approximately 1,000 feet downstream of confluence of Davenport Creek Tributary No. 1	5.54	389	578	663	871
At Oak Island Road	0.40	9	18	21	178
DAVENPORT CREEK TRIBUTARY NO. 1					
Approximately 100 feet downstream of North Goodman Road	3.96	95	224	348	693

Table 5: Summary of Discharges (continued)

		Peak Discharge (Cubic Feet per Second)			
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
DAVENPORT CREEK TRIBUTARY NO. 2			and the second of the second o	the contract that is a strength of the strengt	
Approximately 0.9 mile upstream of mouth	1.56	679	1,066	1,239	1,626
EAST CITY CANAL ¹					
At mouth	6.37	1,128	1,531	1,661	2,018
EAST CITY CANAL TRIBUTARY 1					
At confluence with East City Canal	0.9	375	575	687	932
MILL SLOUGH					
At U.S. Route 441	11.6	710	1,040	1,360	2,050
At Mill Slough Road	10.7	660	970	1,300	1,900
PEG HORN SLOUGH					
At mouth	2.28	714	1,003	1,090	1,258
At Neptune Road	2.01	612	840	896	1,008
At Old Landfill entrance road	1.19	351	416	420	427
At Canoe Creek Road	0.46	209	398	465	508
REEDY CREEK					
At Cypress Lake	282.0	3,300	5,000	5,700	6,350
At Lake Russell	264.0	2,700	4,000	4,500	5,100
At U.S. Route 92 bridge	209.0	800	1,100	1,100	1,100

¹Peak discharges computed with UNET (Reference 25)

Table 5: Summary of Discharges (continued)

	Peak Discharge (Cubic Feet per Second)				nd)
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
REEDY CREEK TRIBUTARY NO. 1			W. VV. VV. VV.		
At mouth	1.83	458	656	32	893
At Marigold Avenue	1.63	336	445	484	559
At San Miguel Road	1.04	145	179	194	231
REEDY CREEK TRIBUTARY NO. 2					
At mouth	0.42	241	357	408	511
At Marigold Avenue	0.19	45	55	5	69
REEDY CREEK TRIBUTARY NO. 3					
At mouth	1.02	223	280	299	344
At Doverplum Avenue	0.90	204	253	264	290
At K.O.A. Street	0.09	63	64	64	64
SHINGLE CREEK					
Approximately 0.3 mile downstream of U.S. Route 17	100.15	5,029	7,561	9,256	13,787
At U.S. Route 17	99.53	5,026	7,552	9,275	13,775
At State Route 531	98.99	5,014	7,541	9,268	13,756
Approximately 0.2 mile upstream of State Route 531	98.09	4,989	7,520	9,246	13,721
At Old Tampa Highway	97.19	4,967	7,500	9,227	13,688
Approximately 1.2 miles upstream of CSX Transportation	78.18	4,208	7,005	8,296	11,618

Table 5: Summary of Discharges (continued)

		Peak Discharge (Cubic Feet per Second)			
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
SHINGLE CREEK (continued)		, , , , , , , , , , , , , , , , , , ,	444.0		- 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3
At State Route 530	76.27	4,183	6,966	8,247	11,549
Approximately 1.1 miles upstream of State Route 530	75.14	4,167	6,942	8,218	11,515
Approximately 1.5 miles downstream of Osceola Parkway	71.54	4,121	6,866	8,124	11,353
At Osceola Parkway	64.67	4,079	6,713	7,929	11,041
TRIBUTARY NO. 1					
At CSX Transportation	2.2	2	2	602	2
At Old Lake Wilson Road	0.2	2	2	336	2
WEST BRANCH SHINGLE CREEK ³					
Approximately 0.7 mile upstream of mouth	16.81	2,616	3,790	4,351	5,640
Approximately 650 feet downstream of Poinciana Boulevard	12.19	2,519	3,673	4,223	5,456
Approximately 0.7 mile downstream of Camelot Country Way	4.52	1,952	2,726	3,033	3,790
Approximately 0.2 mile upstream of Scott Boulevard	1.53	759	1,039	1,153	1,415

² Data Not Available

³ Includes West Branch Shingle Creek Tributary listed separately in previous FIS reports

Table 5: Summary of Discharges (continued)

	Peak Discharge (Cubic Feet per Second)				ond)
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
WEST BRANCH SHINGLE CREEK (continued)				and the second s	tables - in an anni contra con
Approximately 200 feet downstream of mall entrance	0.48	216	296	332	411
WEST CITY CANAL ¹					
At mouth	6.37	1,107	1,418	1,549	1,854
WPA CANAL					
At mouth	13.61	1,196	1,479	1,749	2,769
Approximately 0.8 mile downstream of Florida Turnpike	6.45	580	871	982	1,268
At Florida Turnpike	5.20	357	374	429	613
Approximately 700 feet upstream of Old Canoe Creek Road	3.91	359	460	527	665
At Canoe Creek Road	3.22	250	331	362	454
Approximately 650 feet downstream of West New Nolte Road	2.31	123	168	185	305
At West New Nolte Road	0.86	116	158	175	224
Approximately 0.3 mile upstream of West New Nolte Road	0.69	97	105	108	120

¹Peak discharges computed with UNET (Reference 25)

Table 5: Summary of Discharges (continued)

13.11.11.11.11.11.11.11.11.11.11.11.11.1		Peak Discharge (Cubic Feet per Second)			
Flooding Source and Location	Drainage Area (Square Miles)	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance
WPA CANAL TRIBUTARY 1			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		7. 11. 7. 11.0000 and 4.
At confluence with WPA Canal	4.4	166	317	399	695
Just upstream of confluence of WPA Canal Tributary 1-1	3.4	138	265	334	584
Approximately 1.7 miles upstream of confluence of WPA Canal Tributary 1-1	1.7	84	162	205	363
WPA CANAL TRIBUTARY 1-1					
At confluence with WPA Canal Tributary 1	0.7	137	253	312	488

¹Peak discharges computed with UNET (Reference 25)
² Data Not Available

³ Includes West Branch Shingle Creek Tributary listed separately in previous FIS reports

Table 6: Summary of Stillwater Elevations

		Elevation (feet NAVD88)			
Flooding Source and Location	10-percent- annual-chance	2-percent- annual-chance	1-percent- annual-chance	0.2-percent- annual-chance	
ALLIGATOR LAKE	64.4	65.2	65.5	66.1	
BRICK LAKE	64.5	65.2	65.5	66.2	
BROWN LAKE	63.1	63.7	64.0	64.8	
BUCK LAKE	*	*	104.3	107.5	
CLAY HOLE POND	*	*	65.5	*	
COON LAKE	64.5	65.3	65.6	66.2	
EAGLE POND	*	*	65.4	*	
EAST LAKE TOHOPEKALIGA	57.6	59.5	61.0	63.0	
LAKE BULLOCK	64.6	65.4	65.7	66.3	
LAKE CECILE	75.4	75.9	76.2	77.0	
LAKE CENTER	64.6	65.4	65.7	66.3	
LAKE DAVENPORT	111.0	111.6	112.1	112.7	
LAKE GENTRY	64.3	65.4	65.8	66.7	
LAKE HATCHINEHA	*	*	55.8	*	
LAKE JOEL	62.1	62.9	63.2	64.0	
LAKE KISSIMMEE	52.4	*	54.3	*	
LAKE LIZZIE	64.4	65.2	65.6	66.1	
LAKE MARIAN	*	*	59.1	*	
LAKE MARY JANE	54.9	56.5	57.1	58.6	
LAKE MYRTLE	62.1	62.9	63.2	64.0	
LAKE PRESTON	62.1	62.9	63.2	64.0	
LAKE RUNNYMEDE	57.6	59.5	61.0	63.0	
LAKE TOHOPEKALIGA	54.9	56.5	57.1	58.6	
LAKE WILSON	105.2	106.2	106.5	107.5	
LIVE OAK LAKE	64.4	69.2	65.6	66.1	
OTTER POND	*	*	68.7	*	
PEARL LAKE	64.5	65.2	65.5	66.2	
*D-4 1-1-1-					

^{*}Data not available

Table 6: Summary of Stillwater Elevations (continued)

Flooding Source and Location	10-percent- annual-chance	Elevation (f 2-percent- annual-chance	eet NAVD88) 1-percent- annual-chance	0.2-percent- annual-chance
SARDINE LAKE	64.4	65.2	65.6	66.1
TROUT LAKE	64.5	65.3	65.6	66.2

^{*}Data not available

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Except where noted, cross sections were obtained from field surveys. All bridges, dams, and culverts were field surveyed to obtain elevation data and structural geometry. Cross sections were located at close intervals upstream and downstream of bridge and culverts to compute the significant backwater effects of these structures.

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross section locations are also shown on the FIRM (Exhibit 2).

Table 7, "Manning's 'n' Values," contains the channel and overbank "n" values for the streams studied by detailed methods.

Table 7: Manning's "n" Values

Flooding Source	Channel "n"	Overbank "n"
Bass Slough	0.025	0.025-0.10
Bass Slough Tributary	0.025	0.035-0.075
Big Wateree Creek Tributary 4	*	*
Boggy Creek	0.030	0.020
C-33 Canal	0.025-0.17	0.03-0.18
Canoe Creek (C-34 Canal)	0.025-0.17	0.03-0.18
Davenport Creek	0.025-0.17	0.03-0.18
Davenport Creek Tributary No. 1	0.025-0.17	0.03-0.18

Table 7: Manning's "n" Values (continued)

Flooding Source	Channel "n"	Overbank "n"
Davenport Creek Tributary No. 2	0.025-0.17	0.03-0.18
East City Canal	0.025-0.17	0.03-0.18
East City Canal Tributary 1	0.025-0.11	0.035-0.11
Mill Slough in City of Kissimmee	0.04	0.50
Mill Slough in Osceola County (Unincorporated Areas)	0.030	0.020
Peg Horn Slough	0.025-0.17	0.03-0.18
Reedy Creek	0.030	0.020
Reedy Creek Tributary No. 1	0.025-0.17	0.03-0.18
Reedy Creek Tributary No. 2	0.025-0.17	0.03-0.18
Reedy Creek Tributary No. 3	0.025-0.17	0.03-0.18
Shingle Creek	0.025-0.17	0.03-0.18
St. Johns River	0.025-0.17	0.03-0.18
West Branch Shingle Creek*	0.025-0.17	0.03-0.18
West City Canal	0.025-0.17	0.03-0.18
WPA Canal	0.025-0.17	0.03-0.18
WPA Canal Tributary 1	0.025-0.095	0.025-0.095
WPA Canal Tributary 1-1	0.025	0.045-0.095

^{*}Includes West Branch Shingle Creek Tributary listed separately in previous FIS reports

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals.

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the Flood Profiles (Exhibit 1) are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

3.2.1 Methods for Flooding Sources with New or Revised Analyses in Current Study

Water-surface profiles for the 10-, 2-, 1- and 0.2-percent-annual-chance recurrence intervals were computed for detailed analyses, and the water-surface profile for the 1-percent-annual-chance recurrence interval was computed for approximate analyses. The USACE HEC-RAS step-backwater computer program version 4.0 was utilized for hydraulic analyses of Bass Slough (Lower Reach), Bass Slough (Upper Reach), Bass Slough Tributary, WPA Canal Tributary 1,

WPA Canal Tributary 1-1, East City Canal Tributary 1 and all approximately study streams (Reference 26).

Hydraulic cross section geometries were obtained from the Digital Terrain Model (DTM) and supplemented with surveyed hydraulic data at various locations throughout the detailed studied reaches. All hydraulic structures were field surveyed for detailed analyses. The Watershed Information System (WISE) software was used for preprocessing HEC-RAS data (Reference 27). No floodway was calculated for streams studied by approximate methods.

Starting conditions for the hydraulic models were set to normal depth using starting slopes calculated from channel elevation values taken from the DTM, or set based on effective water-surface elevations as appropriate. Manning's n-values were estimated using high-resolution imagery and field photos for both channel and overbank areas.

The Interconnected Channel and Pond Routing (ICPR) model was used to determine the 1-percent-annual-chance flood elevations for Lake Marian and ponding areas located on its northern and eastern side. The ICPR model was also used to determine the 1-percent-annual-chance flood elevations for ponding areas located west of Reedy Creek and south of Reedy Creek Tributary No. 2. ICPR basins were delineated based on topographic maps with a contour interval of 1 foot. Dimensions of the cross drainage structures at basin boundaries were obtained from field survey. Land use data was obtained from South Florida Water Management District and soil data was obtained from Natural Resources Conservation Service's Soil Data Mart.

3.2.2 Methods for Flooding Sources Incorporated from Previous Studies

Precountywide Analyses

For each jurisdiction within Osceola County that has a previously printed FIS report, the hydraulic analyses described in those reports has been compiled and is summarized below.

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 step-backwater computer program (Reference 28). Starting water-surface elevations were based on the computed water-surface elevation for the receiving stream or lake.

For Reedy Creek, C-1 Canal, C-2 Canal, and C-3 Canal, 1-percent-annual-chance water-surface elevations were determined using the UNET hydraulic model.

In the City of Kissimmee, the hydrologic analyses for the area generally bounded to the north by Vine Street, to the east by Robinson Street, to the south by Sumner Street, and to the west by Main Street determined that the flooded area is subject to shallow flooding where average depths of flooding are less than 3 feet.

Roughness factors (Manning's "n") used in the hydraulic computations were chosen by engineering judgment and were based on field inspection of the floodplain areas.

Revised Analyses for Countywide FIS

Cross sections were obtained from a variety of sources. The primary source was new field surveys. Other cross sections were obtained from the South Florida Water Management District and from the previous FISs.

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE UNET one-dimensional, unsteady flow and HEC-2 water-surface profile computer programs; UNET was used for C-33 Canal, Canoe Creek (C-34 Canal), East City Canal, West City Canal, and Lakes Center, Gentry, Joel, Lizzie, Myrtle, and Preston and Alligator, Brick, Coon, and Trout Lakes; HEC-2 was used for Davenport Creek, Davenport Creek Tributary No. 1, Davenport Creek Tributary No. 2, Peg Horn Slough, Reedy Creek Tributary No. 1, Reedy Creek Tributary No. 2, and Reedy Creek Tributary No. 3, Shingle Creek, West Branch Shingle Creek, West Branch Shingle Creek Tributary, and WPA Canal (References 25 and 28). For the St. Johns River, the hydraulic analyses were taken from *The Mean Annual*, 10-Year, 25-Year, and 100-Year Flood Profiles for the Upper St. Johns River Under Existing Conditions (Reference 19). Starting water-surface elevations for streams were taken to be normal depth; for lakes, the highest operating elevations specified by the South Florida Water Management District were used (Reference 19).

Gage data for historical storm events was used for calibration and verification of the UNET and HEC-2 models. Gage data were obtained from the South Florida Water Management District and the USGS (References 29 and 30). USGS gages were used for the Alligator Chain of Lakes at the S-60 spillway on the C-33 Canal (ID 02260800); at the S-57 culvert on the C-30 Canal (ID 02261500); on the east shore of Cypress Lake near the mouth of Canoe Creek (ID 02266600); on Shingle Creek at the Kissimmee Airport (ID 02263800); on Shingle Creek at Campbell (ID 02264495); and on Davenport Creek near Loughman (ID 02266480). South Florida Water Management District gages were used on the Kissimmee East-West Canal (ID KISSD-H and KISSD-E).

3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum used for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD). With the completion of the North American Vertical Datum of 1988 (NAVD), many FIS reports and FIRMs are now prepared using NAVD as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to the NAVD. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. It is important to note that adjacent counties may be referenced to NGVD, which may result in differences in base flood elevations across county lines.

Ground, structure, and flood elevations may be compared and/or referenced to NGVD29 by applying a standard conversion factor. The conversion factor to NGVD29 is +0.9. This value is an average for the entire county.

The BFEs shown on the FIRM represent whole-foot rounded values. For example, a BFE of 102.4 feet will appear as 102 feet on the FIRM and 102.6 will appear as 103. Therefore, users who wish to convert the elevations in this FIS report to NGVD should apply the stated conversion factor to elevations shown on the Flood Profiles and supporting data tables in the FIS report, which are shown at a minimum to the nearest 0.1 foot.

For more information regarding conversion between the NGVD and NAVD, see the FEMA publication entitled *Converting the National Flood Insurance Program to the North American Vertical Datum of 1988* (Reference 31), visit the National Geodetic Survey website at www.ngs.noaa.gov, or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with the FIS report and FIRM for this community. Interested individuals may contact FEMA to access these data.

To obtain current elevation, description, and/or location information for benchmarks shown on this map, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their website at www.ngs.noaa.gov.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS report provides 1-percent-annual-chance floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2-percent-annual-chance flood elevations; delineations of the 1- and 0.2-percent-annual-chance floodplains; and a 1-percent-annual-chance floodway. This information is presented on the FIRM and in many components of the FIS report, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS report as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community. For each stream studied by detailed or limited detailed methods, the 1- and 0.2-percent-annual-chance floodplain boundaries have been delineated using the flood elevations determined at each cross section.

Between cross sections, the boundaries were interpolated based on contours developed from the Digital Terrain Model (Reference 32).

The 1- and 0.2-percent-annual-chance floodplain boundaries for streams studied by detailed methods are shown on the FIRM. On this map, the 1-percent-annual-chance floodplain boundary corresponds to the boundary of the areas of special flood hazards (Zones A and AE), and the 0.2-percent-annual-chance floodplain boundary corresponds to the boundary of areas of moderate flood hazards. In cases where the 1- and 0.2-percent-annual-chance floodplain boundaries are close together, only the 1-percent-annual-chance floodplain boundary has been shown. Small areas within the floodplain boundaries may lie above the flood elevations, but cannot be shown due to limitations of the map scale and/or lack of detailed topographic data.

For the flooding sources studied by approximate methods, only the 1-percent-annual-chance floodplain boundary is shown on the FIRM (Exhibit 2).

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the base flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this study were computed for certain stream segments on the basis of equal-conveyance reduction from each side of the floodplain. Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections and provided in Table 8, "Floodway Data." The computed floodway is shown on the FIRM (Exhibit 2). In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown on the FIRM.

No floodway was computed Davenport Creek Tributary 1, Davenport Creek Tributary 2, St. Johns River, and Tributary No. 1.

Near the confluence of streams studied in detail, floodway computations were made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 8, "Floodway Data," for certain downstream cross sections of Bass Slough Tributary, East City Canal Tributary 1, Reedy Creek Tributary No. 1, Reedy Creek Tributary No. 2, Shingle Creek, West City Canal, WPA Canal Tributary 1, and WPA Canal Tributary 1-1 are lower than the regulatory flood elevations in that area, which must take into account the 1-percent-annual-chance flooding due to backwater from other sources.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage and heightens potential flood hazards by further increasing velocities. A listing of stream velocities at selected cross sections is provided in Table 8. To reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation (WSEL) of the base flood more than 1 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 1.

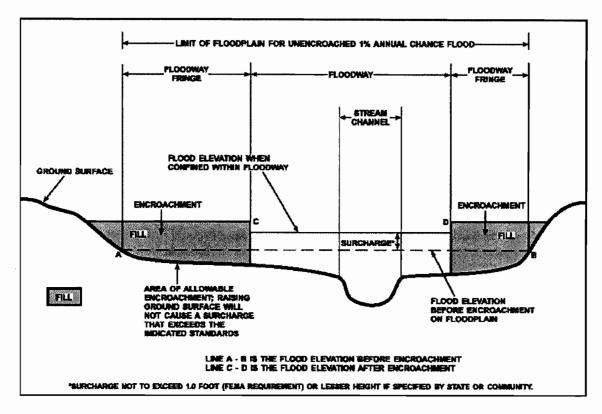


Figure 1. Floodway Schematic

CROSS SECTION DISTANCE (F) BASS SLOUGH (LOWER REACH) A 2,000 B 2,999 C 4,500 D 6,000 E 7,000 F 7,500 G 9,000 H 10,500	SECTION AREA (SQUARE FEET) 58 200 320 1,501 500 1,677 235 618 143 487 62 201	MEAN VELOCITY (FEET PER SECOND) 5.9 0.8 0.6 1.7 2.2 5.3	FEGULATORY (FEET NAVD 88) 58.5 60.8 60.9 61.1 63.0	WITHOUT FLOODWAY (FEET NAVD 88) 58.5 60.8 60.9 61.1 63.0	WITH FLOODWAY (FEET NAVD 88) 59.2 61.1 61.3 61.7	0.7 0.3 0.4 0.6
(LOWER REACH) A 2,000 B 2,999 C 4,500 D 6,000 E 7,000 F 7,500 G 9,000 H 10,500	320 1,501 500 1,677 235 618 143 487	0.8 0.6 1.7 2.2	60.8 60.9 61.1	60.8 60.9 61.1	61.1 61.3 61.7	0.3 0.4
B 2,999 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	320 1,501 500 1,677 235 618 143 487	0.8 0.6 1.7 2.2	60.8 60.9 61.1	60.8 60.9 61.1	61.1 61.3 61.7	0.3 0.4
C 4,500 5 6,000 2 6 7,000 6 7,500 G 9,000 4 10,500	500 1,677 235 618 143 487	0.6 1.7 2.2	60.9 61.1	60.9 61.1	61.3 61.7	0.4
D 6,000 2 E 7,000 7 F 7,500 9,000 4 H 10,500	235 618 143 487	1.7 2.2	61.1	61.1	61.7	
E 7,000 7 F 7,500 9,000 4 H 10,500	143 487	2.2			l l	0.6
F 7,500 G 9,000 H 10,500			63.0	63.0	ا ممم	
F 7,500 G 9,000 H 10,500	62 201	5.3		03.0	63.8	0.8
H 10,500		0.0	63.5	63.5	64.1	0.6
	450 1,939	0.6	64.3	64.3	64.7	0.4
I 12,655	93 272	3.9	64.4	64.4	64.8	0.4
	324 636	1.3	67.3	67.3	67.9	0.6
J 14,008 2	268 1,432	0.6	68.9	68.9	69.7	0.8
K 15,003	94 128	6.5	70.2	70.2	70.2	0.0
L 16,508	129 223	2.6	74.4	74.4	74.6	0.2
M 18,008 2	204 374	1.5	74.7	74.7	75.0	0.3
N 19,008 2	283 203	2.8	75.7	75.7	76.1	0.4

¹ Stream distance in feet above mouth

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

BASS SLOUGH (LOWER REACH)

FLOODING S	OURCE		FLOODWA	AY	BASI	FLOOD WA	TER SURFA	CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
BASS SLOUGH (UPPER REACH)								
Α	21,799	70	253	2.1	78.8	78.8	79.1	0.3
В	23,008	182	658	0.8	79.3	79.3	79.8	0.5
С	24,008	283	1,113	0.3	79.4	79.4	79.8	0.4
D	25,508	152	491	0.6	79.4	79.4	79.9	0.5
E	26,508	124	276	1.1	79.4	79.4	79.9	0.5
F	27,008	139	422	0.7	79.5	79.5	79.9	0.4
G	28,008	128	321	0.9	79.5	79.5	80.1	0.6
н	28,791	76	217	1.3	79.5	79.5	80.1	0.6

¹ Stream distance in feet above mouth

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

BASS SLOUGH (UPPER REACH)

FLOODING S	OURCE		FLOODWA	AY	BASI	FLOOD WA	TER SURFA	CE
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
BASS SLOUGH TRIBUTARY								
Α	217 ¹	50	91	2.0	79.4	75.8 ²	76.8	1.0
В	1,000 ¹	30	90	2.0	79.4	77.4 ²	77.9	0.5
С	2,040 ¹	50	200	0.9	79.4	78.1 ²	79.0	0.9
BOGGY CREEK								
Α	5,600 ³	1,020	2,900	1.8	61.0	61.0	62.0	1.0

¹ Stream distance in feet above confluence with Bass Slough

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

BASS SLOUGH TRIBUTARY & BOGGY CREEK

² Elevation computed without consideration of backwater effects from Bass Slough

³ Stream distance in feet above confluence with East Lake Topopekaliga

FLOODING S	OURCE	FLOODWAY			BASI	E FLOOD WA ELEVA		CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
C-33 CANAL			•					
Α	380	212	545	0.3	65.5	65.5	65.5	0.0
В	2,920	243	459	0.3	65.5	65.5	65.5	0.0
С	3,770	17	222	0.3	65.8	65.8	65.8	0.0
D	6,020	129	784	0.1	65.8	65.8	65.8	0.0
E	8,620	125	964	0.1	65.8	65.8	65.8	0.0
F	11,405	251	1,133	0.1	65.8	65.8	65.8	0.0
G	12,405	245	1,165	0.1	65.8	65.8	65.8	0.0

¹ Stream distance in feet above confluence with Alligator Lake ² At time of peak water-surface elevation

TABLE ∞

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

C-33 CANAL

OURCE		FLOODWAY					CE
DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
0	123	998	3.4	56.1	56.1	56.1	0.0
450	37	988	3.4	64.1	64.1	64.1	0.0
5,628	88	1,506	0.3	64.6	64.6	64.6	0.0
15,389	20	346	1.4	65.8	65.8	65.8	0.0
	0 450 5,628	DISTANCE ¹ WIDTH (FEET) 0 123 450 37 5,628 88	DISTANCE ¹ WIDTH (FEET) SECTION AREA (SQUARE FEET) 0 123 998 450 37 988 5,628 88 1,506	DISTANCE1 WIDTH (FEET) SECTION AREA (SQUARE FEET) MEAN VELOCITY (FEET PER SECOND) 0 123 998 3.4 450 37 988 3.4 5,628 88 1,506 0.3	DISTANCE	DISTANCE WIDTH (FEET) SECTION AREA (SQUARE FEET) SECOND) WITHOUT FLOODWAY (FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88) SECOND SECOND	DISTANCE WIDTH (FEET) SECTION AREA (SQUARE FEET) FEET SECOND) REGULATORY (FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88) FEET NAVD 88) FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88) FEET NAVD 88) FEE

¹ Stream distance in feet above Canoe Creek Road

² At time of peak water-surface elevation

TABLE 8

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

CANOE CREEK (C-34 CANAL)

FLOODING S	OURCE		FLOODWA	AY	BASI	E FLOOD WA ELEVA	TER SURFA	CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
DAVENPORT CREEK								
Α	4,500	734	3,963	1.0	80.2	80.2	81.2	1.0
В	5,330	440 ²	2,627	1.5	81.8	81.8	82.4	0.6
С	6,730	359	2,112	1.7	84.4	84.4	84.8	0.4
D	8,130	292	1,723	2.1	86.0	86.0	86.8	0.8
E	9,230	112	794	4.6	88.9	88.9	89.5	0.6
F	10,520	223	2,087	1.4	92.9	92.9	93.6	0.7
G	11,820	231	1,596	1.9	93.5	93.5	94.2	0.7
Н	13,385	136	819	3.6	95.1	95.1	95.5	0.4
1	14,045	42	453	6.5	96.2	96.2	96.7	0.5
J	15,745	191	1,782	1.7	98.8	98.8	99.6	0.8
K	16,872	384	3,259	0.9	102.6	102.6	103.2	0.6
L	18,342	228	1,923	1.5	103.2	103.2	103.8	0.6
M	20,392	286	1,627	1.7	104.9	104.9	105.6	0.7
N-R*								

TABLE ∞

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

DAVENPORT CREEK

¹ Stream distance in feet above confluence with Reedy Creek
² Value represents total width; however, floodway is not shown inside Reedy Creek Improvement District

^{*} Floodway data not available

CROSS SECTION D	DISTANCE ¹	WIDTH	SECTION AREA	MEAN		D 88) FLOODWAY FLOODWAY (F			
		(FEET)	(SQUARE FEET)	VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)			INCREASE (FEET)	
EAST CITY CANAL									
Α	3,374	76	447	3.5	57.8	57.8	57.9	0.1	
В	4,124	96	363	4.2	58.5	58.5	58.5	0.0	
С	4,932	90	414	3.7	61.1	61.1	61.1	0.0	
D	5,433	80	409	3.7	61.5	61.5	61.5	0.0	
E	8,232	100	62	2.3	65.1	65.1	65.1	0.0	
F	8,733	170	807	1.4	65.3	65.3	65.3	0.0	
G	10,127	250 ³	1,119	1.0	65.9	65.9	65.9	0.0	
Н	10,629	265 ³	1,799	0.5	66.0	66.0	66.0	0.0	
1	11,278	250 ³	1,611	0.5	66.0	66.0	66.0	0.0	
J	11,980	250	1,292	0.6	66.0	66.0	66.0	0.0	
K	13,158	75	381	2.0	66.2	66.2	66.3	0.1	
L	14,362	60	291	0.2	66.5	66.5	66.7	0.2	
М	15,729	41	108	0.3	66.9	66.9	66.9	0.0	
N	16,516	41	96	0.5	67.7	67.7	67.7	0.0	

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FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

EAST CITY CANAL

¹ Stream distance in feet above confluence with Lake Tohopekaliga
² At time of peak water-surface elevation
³ Value is inaccurate, as the floodway has been adjusted in this area to match topographic-based floodplain redelineation

OURCE		FLOODWA	AY	BASI			CE
DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
		•	•				
500	42	274	2.5	65.1	63.7 ²	64.4	0.7
1,000	46	233	3.0	65.1	63.8 ²	64.5	0.7
2,000	93	763	0.9	66.0	66.0	67.0	1.0
2,373	66	577	1.2	66.0	66.0	67.0	1.0
				·			
	500 1,000 2,000	DISTANCE ¹ WIDTH (FEET) 500 42 1,000 46 2,000 93	DISTANCE ¹ WIDTH (FEET) SECTION AREA (SQUARE FEET) 500 42 274 1,000 46 233 2,000 93 763	DISTANCE ¹ WIDTH (FEET) SECTION AREA (SQUARE FEET) WELOCITY (FEET PER SECOND) 500 42 274 2.5 1,000 46 233 3.0 2,000 93 763 0.9	DISTANCE	DISTANCE	DISTANCE WIDTH (FEET) SECTION AREA (SQUARE FEET) FEET) REGULATORY (FEET NAVD 88) REGULATORY (FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88) FEET NAVD 88) FEET NAVD 88)

¹ Stream distance in feet above confluence with East City Canal

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

EAST CITY CANAL TRIBUTARY 1

² Elevation computed without consideration of backwater effects from East City Canal

FLOODING S	OURCE		FLOODWA	AY	BASI	E FLOOD WA ELEVA	TER SURFA	CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
MILL SLOUGH								
A	6,161	90	460	3.0	57.1	57.1	58.1	1.0
В	7,090	140	450	3.0	57.9	57.9	58.6	0.7
С	9,090	230	830	1.6	59.5	59.5	60.3	0.8
D	10,590	50	250	5.5	60.5	60.5	61.5	1.0
E	11,391	40	250	5.2	64.0	64.0	64.5	0.5
F	13,350	202	606	2.1	67.3	67.3	68.2	0.9
G	14,410	45	248	5.2	69.1	69.1	69.8	0.7
н	15,450	59	295	4.4	72.3	72.3	72.5	0.2
1	17,900	81	438	3.0	76.2	76.2	76.7	0.5
J	20,150	181	695	1.9	79.1	79.1	79.9	0.8

¹ Stream distance in feet above confluence with Lake Tohopekaliga

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

MILL SLOUGH

FLOODING S	OURCE	FLOODWAY		ΑY	BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
PEG HORN SLOUGH								
A	1,300	34	215	5.1	60.7	60.7	61.2	0.5
В	3,022	205	879	1.2	62.4	62.4	62.7	0.3
С	5,432	47	413	2.2	63.0	63.0	63.5	0.5
D	7,200	55	517	1.7	65.4	65.4	66.1	0.7
E	9,491	56	524	0.8	66.1	66.1	67.0	0.9
F	9,831	21	188	2.2	66.1	66.1	67.0	0.9
G	10,346	46	420	1.0	66.7	66.7	67.6	0.9
Н	14,436	55	524	0.9	68.0	68.0	68.9	0.9
l I	14,751	22	198	2.3	68.0	68.0	68.8	0.8
J	15,575	54	471	1.0	69.8	69.8	70.6	0.8
K	16,150	46	243	1.9	69.8	69.8	70.6	0.8
L	17,117	444	1,587	0.3	71.6	71.6	71.9	0.3

¹ Stream distance in feet above confluence with St. Cloud Canal (Canal 31)

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

PEG HORN SLOUGH

FLOODING S	OURCE		FLOODWA	AY	BASI	E FLOOD WA ELEVA		CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
REEDY CREEK				_				
Α	3.55	5,568	23,325	0.2	57.7	57.7	58.7	1.0
В	4.73	5,455	12,337	0.4	58.7	58.7	59.6	0.9
С	5.95	2,754	14,138	0.4	60.4	60.4	61.4	1.0
D	6.74	1,513	11,487	0.4	60.7	60.7	61.7	1.0
E F	8.81	1,514	10,628	0.4	62.3	62.3	63.3	1.0
F	9.45	1,384	9,590	0.4	63.0	63.0	64.0	1.0
G	9.79	712	5,325	0.7	63.3	63.3	64.3	1.0
Н	10.89	1,306	9,011	0.4	64.3	64.3	65.3	1.0
I	11.98	2,471	15,454	0.2	64.7	64.7	65.7	1.0
J	13.05	2,495	14,137	0.2	65.1	65.1	66.1	1.0
K	14.07	3,192	15,140	0.2	65.4	65.4	66.4	1.0
L	15.10	5,081	24,716	0.1	65.6	65.6	66.6	1.0
M	16.12	2,918	15,285	0.2	65.8	65.8	66.8	1.0
N	16.67	2,040	8,894	0.2	66.0	66.0	67.0	1.0
0	17.17	4,644	20,654	0.1	66.1	66.1	67.1	1.0
Р	18.18	4,640	19,641	0.1	66.2	66.2	67.2	1.0
Q	19.35	7,514	17,744	0.1	66.4	66.4	67.4	1.0
R	20.33	84	307	3.6	67.0	67.0	67.9	0.9

¹ Stream distance in miles above confluence with Cypress Lake

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TABLE

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

REEDY CREEK

FLOODING S	OURCE		FLOODWA	AY	BASI	E FLOOD WA	TER SURFA	CE
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
REEDY CREEK TRIBUTARY NO. 1								
Α	5,650	51	172	4.3	65.3	62.2 ²	63.2	1.0
В	6,620	64	217	2.2	65.3	63.9 ²	64.5	0.6
С	10,678	599	1,462	0.1	67.5	67.5	67.7	0.2
D	12,468	1,200	3,179	0.1	67.6	67.6	67.8	0.2
REEDY CREEK TRIBUTARY NO. 2								
Α	1,950	69	192	2.1	64.8	62.4 ²	63.4	1.0
В	2,870	46	134	3.0	65.2	65.2	65.4	0.2
С	4,020	33	86	0.7	67.0	67.0	67.0	0.0
D	5,356	533	96	0.1	67.4	67.4	67.4	0.0

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS

FLOODWAY DATA

REEDY CREEK TRIBUTARY NO. 1 & NO. 2

¹ Steam distance in feet above confluence with Reedy Creek ² Elevation computed without consideration of backwater effects from Reedy Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
REEDY CREEK TRIBUTARY NO. 3								
Α	4,377	67	271	1.0	65.6	65.6	66.0	0.4
В	7,197	82	343	0.8	65.9	65.9	66.2	0.3
С	8,497	191	390	0.7	66.0	66.0	66.3	0.3
D	9,787	991	1,885	0.1	66.1	66.1	66.4	0.3
E	10,987	197	422	0.2	66.2	66.2	66.5	0.3
F	12,507	39	82	0.8	67.6	67.6	67.7	0.1
G	12,927	36	56	1.1	67.7	67.7	67.8	0.1

¹ Stream distance in feet above confluence with Reedy Creek

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

REEDY CREEK TRIBUTARY NO. 3

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
SHINGLE CREEK			•			-		
Α	5,100	1,280	4,584	2.0	57.1	56.3 ²	57.3	1.0
В	9,230	675	3,762	2.5	61.3	61.3	62.2	0.9
С	11,002	734	3,944	1.6	62.7	62.7	63.5	0.8
D	14,726	1,713	12,828	0.7	64.9	64.9	65.6	0.7
E	19,856	717	5,258	1.6	65.7	65.7	66.7	1.0
E F	33,578	805	5,479	1.5	73.0	73.0	73.6	0.6
G	35,878	1,600	11,425	0.7	73.4	73.4	74.0	0.6
Н	43,128	1,099	7,583	1.1	75.4	75.4	76.2	0.8

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

SHINGLE CREEK

¹ Stream distance in feet above confluence with Lake Tohopekaliga ² Elevation computed without consideration of backwater effects from Lake Tohopekaliga

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
WEST BRANCH SHINGLE CREEK								
Α	3,733	2,092	5,393	0.8	64.9	63.0 ²	63.5	0.5
В	6,750	1,077	4,123	1.0	64.9	64.8 ²	65.3	0.5
С	9,553	746	4,108	1.0	65.5	65.5	66.2	0.7
D	10,139	256	1,608	2.6	66.4	66.4	67.0	0.6
E	14,039	3,529	26,114	0.1	67.4	67.4	68.0	0.6
F	18,449	3,029	17,973	0.2	67.5	67.5	68.0	0.5
G	22,831	2,100	11,888	0.3	67.8	67.8	68.8	1.0
Н	24,951	2,509	26,545	0.1	67.8	67.8	68.8	1.0
1	30,072	1,320	6,432	0.4	68.0	68.0	69.0	1.0
J	30,677	465	2,811	1.0	68.0	68.0	69.0	1.0
K	31,607	418	2,000	0.6	68.2	68.2	69.2	1.0
L	32,287	457	2,462	0.5	68.3	68.3	69.3	1.0
M	33,647	204	641	1.8	68.7	68.7	69.5	0.8
N	34,658	8	34	9.7	71.0	71.0	71.5	0.5

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

WEST BRANCH SHINGLE CREEK

¹ Stream distance in feet above confluence with Shingle Creek
² Elevation computed without consideration of backwater effects from Shingle Creek

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
WEST CITY CANAL								
Α	800	73	360	4.3	57.1	57.0 ²	57.1	0.1
В	1,378	72	641	2.5	57.4	57.4	57.5	0.1
С	2,328	107	610	2.6	57.8	57.8	57.8	0.0
D	4,097	61	351	1.7	59.1	59.1	59.2	0.1
E	4,699	49	253	2.3	59.2	59.2	59.3	0.1
F	5,676	56	313	1.9	61.8	61.8	61.8	0.0
G	6,299	80	382	1.6	62.3	62.3	62.3	0.0
Н	6,674	200	643	0.9	62.3	62.3	62.4	0.1
l	7,593	300	968	0.5	62.8	62.8	62.8	0.0
J	8,432	150	755	0.7	63.1	63.1	63.2	0.1
K	9,641	70	238	1.7	63.7	63.7	63.8	0.1
L	11,458	54	325	0.7	67.6	67.6	67.6	0.0

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FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

WEST CITY CANAL

¹ Stream distance in feet above confluence with Lake Tohopekaliga ² Elevation computed without consideration of backwater effects from Lake Tohopekaliga

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
WPA CANAL								·
Α	750	250	823	2.1	57.9	57.9	58.3	0.4
В	2,875	1,001	3,697	0.5	58.2	58.2	58.7	0.5
С	6,875	1,033	5,716	0.3	58.4	58.4	58.9	0.5
D	12,285	50	201	4.9	63.8	63.8	63.9	0.1
Е	13,350	52	299	1.4	67.7	67.7	68.3	0.6
F	15,608	543	1,324	0.1	67.9	67.9	68.9	1.0
G	17,024	56	269	0.3	68.0	68.0	68.9	0.9
Н	20,476	47	192	1.9	69.1	69.1	69.8	0.7
	21,154	42	143	2.5	69.5	69.5	69.7	0.2
J	21,927	40	173	2.1	70.3	70.3	70.5	0.2
K	22,167	29	126	2.9	70.4	70.4	70.6	0.2
L	22,337	46	236	1.5	70.9	70.9	71.7	0.8
M	24,233	34	109	1.6	71.6	71.6	72.5	0.9
N	24,423	48	170	1.0	72.9	72.9	73.6	0.7
0	28,538	363	397	0.3	73.4	73.4	73.9	0.5

¹ Stream distance in feet above confluence with Lake Tohopekaliga

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

WPA CANAL

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)
WPA CANAL TRIBUTARY 1								
Α	1,500	171	475	0.8	71.4	70.9 ²	71.9	1.0
В	2,500	330	517	0.8	74.2	74.2	75.1	0.9
С	3,500	485	848	0.5	74.7	74.7	75.6	0.9
D	11,000	1,448	4,403	0.1	74.8	74.8	75.8	1.0
E	12,500	1,140	2,027	0.2	74.8	74.8	75.8	1.0
F	14,000	1,269	2,260	0.1	74.8	74.8	75.8	1.0

¹ Stream distance in feet above confluence with WPA Canal

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

WPA CANAL TRIBUTARY 1

² Elevation computed without consideration of backwater effects from WPA Canal

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER SURFACE ELEVATION			
DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY (FEET NAVD 88)	WITHOUT FLOODWAY (FEET NAVD 88)	WITH FLOODWAY (FEET NAVD 88)	INCREASE (FEET)	
		•						
2,000	88	180	1.7	74.8	74.1 ²	74.3	0.2	
4,000	312	425	0.7	74.8	74.6 ²	75.4	0.8	
	DISTANCE ¹ 2,000	DISTANCE ¹ WIDTH (FEET) 2,000 88	DISTANCE ¹ WIDTH (FEET) SECTION AREA (SQUARE FEET) 2,000 88 180	DISTANCE ¹ WIDTH (FEET) SECTION AREA (SQUARE FEET) MEAN VELOCITY (FEET PER SECOND) 2,000 88 180 1.7	DISTANCE SECTION MEAN VELOCITY (FEET PER SECOND) 2,000 88 180 1.7 74.8	DISTANCE SECTION AREA (SQUARE FEET) SECOND) REGULATORY (FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88)	DISTANCE WIDTH (FEET) SECTION AREA (SQUARE FEET) SECOND) REGULATORY (FEET NAVD 88) WITHOUT FLOODWAY (FEET NAVD 88) FEET NAVD 88) FEET NAVD 88)	

¹ Stream distance in feet above confluence with WPA Canal Tributary 1

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **FLOODWAY DATA**

WPA CANAL TRIBUTARY 1-1

² Elevation computed without consideration of backwater effects from WPA Canal Tributary 1

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. These zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS report by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base (1-percent-annual-chance) flood elevations (BFEs) or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS report by detailed methods. Whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2-percent-annual-chance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percent-annual-chance flooding where average depths are less than 1 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1 square mile (sq. mi.), and areas protected from the base flood by levees. No BFEs or depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1- and 0.2-percent-annual-chance floodplains, floodways, and the locations of selected cross sections used in the hydraulic analyses and floodway computations.

The current FIRM presents flooding information for the entire geographic area of Osceola County. Previously, separate Flood Hazard Boundary Maps and/or FIRMs were prepared for each incorporated community and the unincorporated areas of the County identified as flood-prone. This countywide FIRM also includes flood-hazard information that was presented separately on Flood Boundary and Floodway Maps (FBFMs), where applicable. Historical data relating to the maps prepared for each community are presented in Table 9, "Community Map History."

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Kissimmee, City of	July 19, 1974	April 30, 1976	July 2, 1981	May 7, 2001
Osceola County (Unincorporated Areas)	January 31, 1975	August 12, 1977	February 3, 1982	March 16, 1989 November 20, 1996 May 7, 2001
Reedy Creek Improvement District	May 7, 2001	None	May 7, 2001	None
St. Cloud, City of	June 28, 1974	February 20, 1976	September 17, 1980	October 23, 1981 April 3, 1996 May 7, 2001

FEDERAL EMERGENCY MANAGEMENT AGENCY

OSCEOLA COUNTY, FL AND INCORPORATED AREAS **COMMUNITY MAP HISTORY**

7.0 OTHER STUDIES

Flood Hazard Studies for St. Cloud, Kissimmee, and the unincorporated areas of Orange County have been prepared by the USACE, Jacksonville District. The flood elevations determined in this FIS do not conflict with the flood elevations determined for those FISs.

The USACE, Jacksonville District prepared an expanded floodplain information report on the Boggy Creek basin, dated April 1979 (Reference 10). The flood stages in that report agree with the flood stages in this FIS.

The USACE, Jacksonville District, prepared a flood hazard information report on Reedy Creek in June 1976 (Reference 12). The flood stages presented in that report agree with those contained in this FIS.

In June 1973, the USACE published a floodplain information report on Mill Slough (Reference 13). That report included 1-percent-annual-chance and standard project flood profiles for Mill Slough, which agree with the 1-percent-annual-chance flood profiles contained in this FIS.

The USACE, Jacksonville District prepared a reconnaissance report on the Kissimmee River basin, which was published in September 1979 (Reference 33). That report describes the procedures to be used in evaluating the feasibility of modifying the existing flood control system for improving water quality and enhancing fish and wildlife resources in the Kissimmee River basin. The study will update the hydrologic analyses conducted previously by the USACE in the Kissimmee River basin.

Reynolds, Smith, and Hills, Architects-Engineers-Planners for Orange County, Osceola County, South Florida Water Management District, and the Florida Division of State Planning prepared a floodplain information report (Reference 11). The flood stages in that report agree with those contained in this FIS.

FIS reports have been prepared for the unincorporated areas of Okeechobee and Polk Counties and countywide FISs have been prepared for Brevard, Indian River, and Orange Counties (References 34, 35, 36, 37, and 1).

This FIS report supersedes or is compatible with all previous studies published on streams studied in this report and should be considered authoritative for the purposes of the NFIP.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting Federal Insurance and Mitigation Division, FEMA Region IV, Koger-Center — Rutgers Building, 3003 Chamblee Tucker Road, Atlanta, GA 30341.

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- 33. U.S. Army Corps of Engineers, Jacksonville District, Central and Southern Florida, Kissimmee River, Florida, Reconnaissance Report (Stage I), September 1979.
- 34. Federal Emergency Management Agency, Federal Insurance Administration, Flood Insurance Study, Okeechobee County, Florida (Unincorporated Areas), Washington, D.C., August 4, 1980 (FIS report), February 4, 1981 (FIRM).
- 35. Federal Emergency Management Agency, Flood Insurance Study, Polk County. Florida (Unincorporated Areas), Washington, D.C., November 19, 2003 (FIS report), November 19, 2003 (FIRM).
- 36. Federal Emergency Management Agency, Flood Insurance Study, Brevard County, Florida and Incorporated Areas, Washington, D.C., November 19, 1997.
- 37. Federal Emergency Management Agency, Flood Insurance Study, Indian River County, Florida and Incorporated Areas, Washington, D.C., May 4, 1989.

10.0 REVISION DESCRIPTIONS

This section has been added to provide information regarding significant revisions made since the original FIS was printed. Future revisions may be made that do not result in the republishing of the FIS report. To assure that the user is aware of all revisions, it is advisable to contact the community repository of flood-hazard data located at:

- City of Kissimmee
 Engineering Department, Suite 301
 101 North Church Street
 Kissimmee, Florida 34741
- Osceola County (Unincorporated Areas)
 Stormwater Section, Suite 1400
 1 Courthouse Square
 Kissimmee, Florida 34741
- Reedy Creek Improvement District Administrative Building 1900 Hotel Plaza Boulevard Lake Buena Vista, Florida 32830
- St. Cloud City Hall
 Public Works Department, Building A, 2nd Floor
 1300 9th Street
 St. Cloud, Florida 34769

10.1 First Revision June 18, 2013

This June 18, 2013 revision was initiated in support of the FEMA Risk MAP Program.

This revision involved countywide remapping of Osceola County, Florida. The revision included new detailed studies, refinement and establishment of approximate zones,

redelineation of existing studies, and creation of new FIRMs using new topographic and base map data.

